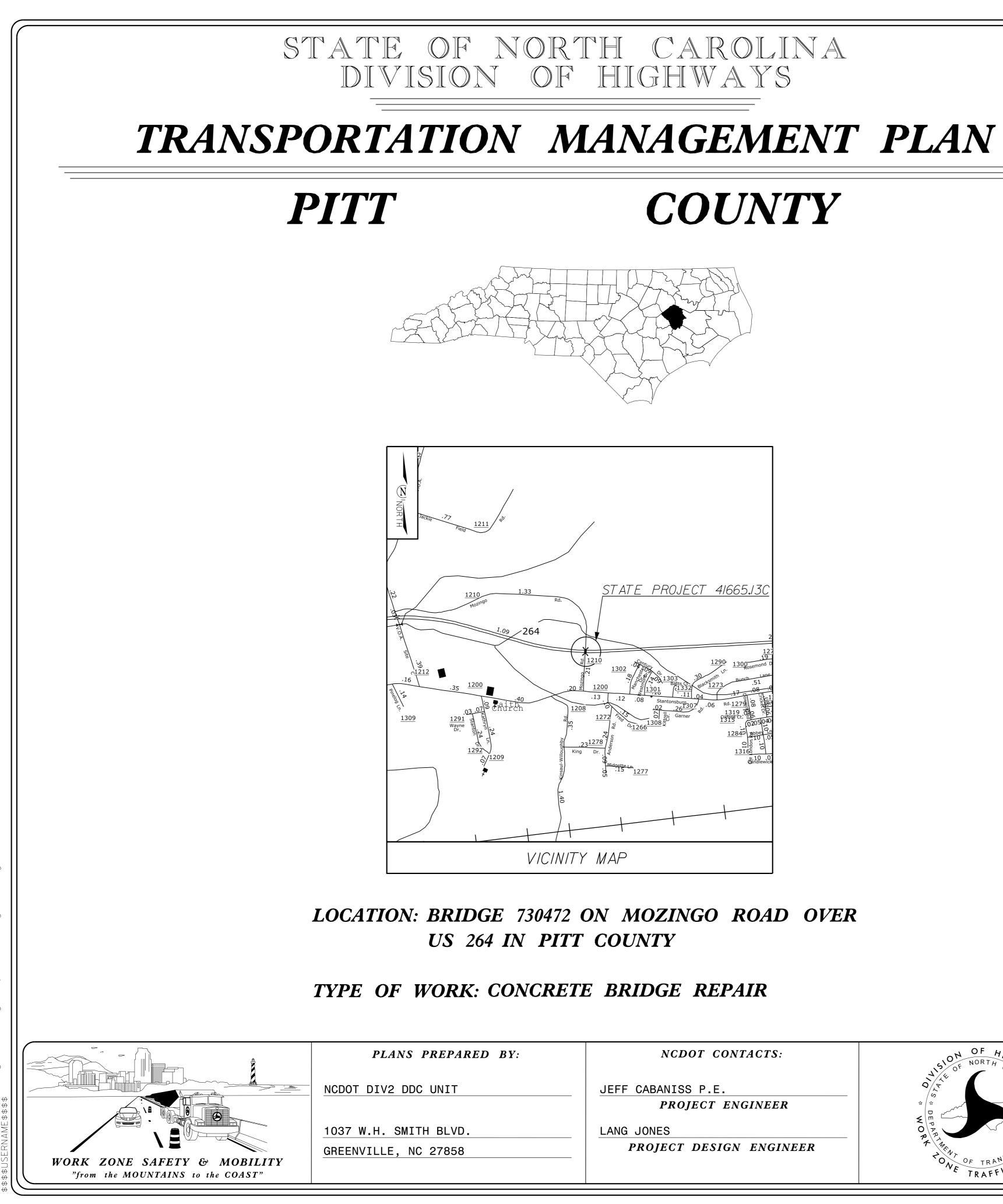


CT 41665.13C	DIVISION	d in the Office of: OF HIGHWAYS Blvd., Greenville NC, 27858
	2018 STANDARD SPECIFICATIONS	
0.019 MILES	<i>RIGHT OF WAY DATE:</i>	JEFF CABANNIS PROJECT ENGINEER
	<i>LETTING DATE:</i> 01/22	LANG JONES PROJECT DESIGN ENGINEER

	STATE	STATE	PROJECT REFERENCE NO.	SHEET TOTAL NO. SHEETS
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SHEET NO.

TMP-1

TMP-1A

TMP-2

TMP-1B

NDEX OF SHEETS	
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TITLE

TITLE SHEET, VICINITY MAP, AND INDEX OF SHEETS LIST OF APPLICABLE ROADWAY STANDARD DRAWINGS, AND LEGEND

TRANSPORTATION OPERATIONS PLAN: (MANAGEMENT STRATEGIES, GENERAL NOTES, AND LOCAL NOTES)

PROJECT PHASING

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SHEET NO.

TMP-1

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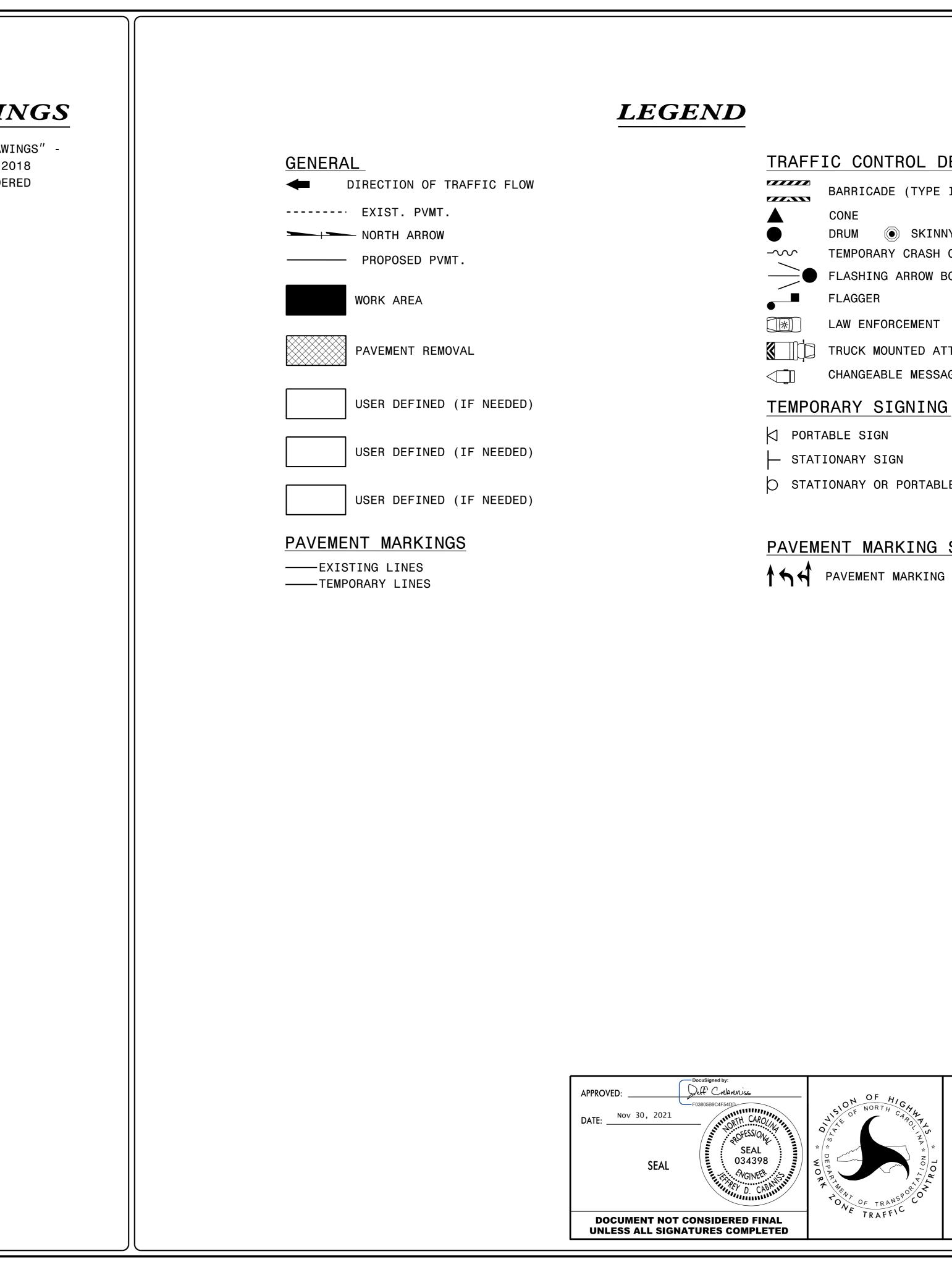
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ROADWAY STANDARD DRAWINGS

THE FOLLOWING ROADWAY STANDARDS AS SHOWN IN "ROADWAY STANDARD DRAWINGS" -N.C. DEPARTMENT OF TRANSPORTATION - RALEIGH, N.C., DATED JANUARY 2018 ARE APPLICABLE TO THIS PROJECT AND BY REFERENCE HEREBY ARE CONSIDERED A PART OF THESE PLANS:

TITLE STD. NO.

1101.02 TEMPORARY LANE CLOSURES 1101.03 TEMPORARY ROAD CLOSURES 1101.11 TRAFFIC CONTROL DESIGN TABLES STATIONARY WORK ZONE SIGNS 1110.01 PORTABLE WORK ZONE SIGNS 1110.02 FLASHING ARROW BOARDS 1115.01 DRUM 1130.01 CONES 1135.01 FLAGGING DEVICES 1150.01 PORTABLE CONCRETE BARRIER (ANCHORED) 1170.01



PROJ. REFERENCE NO.	SHEET NO.
41665.13C	TMP-1A

TRAFFIC CONTROL DEVICES

	BARRICADE (TYPE III)
	CONE
\bullet	DRUM
-~~	TEMPORARY CRASH CUSHION
\rightarrow	FLASHING ARROW BOARD
•	FLAGGER
	LAW ENFORCEMENT
	TRUCK MOUNTED ATTENUATOR (TMA)
	CHANGEABLE MESSAGE SIGN
	RARV STGNING

- STATIONARY OR PORTABLE SIGN

PAVEMENT MARKING SYMBOLS

PAVEMENT MARKING SYMBOLS

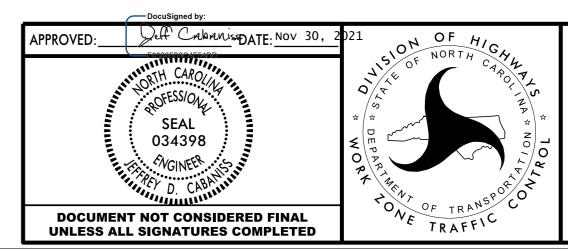
ROADWAY STANDARD DRAWINGS & LEGEND

	CHANGES MAY BE REQUIRED WHE DRAWINGS, STANDARD DETAILS, TO MEET FIELD CONDITIONS OF OVERLAPPING OF DEVICES. MC SUPPLEMENTING, COVERING, OF ENGINEER.	AND ROADWAY DET RESULT IN DUPLI DIFICATION MAY I	AILS ARE NOT ATTAINABLE CATE OR UNDESIRED NCLUDE: MOVING,
	THE FOLLOWING GENERAL NOTES THE CONSTRUCTION PROJECT EX OR DIRECTED BY THE ENGINEER	CEPT WHEN OTHERW	
LANE	E AND SHOULDER CLOSURE REQUIR	REMENTS	
A)	DO NOT STOP TRAFFIC AS FOLLO	WS:	
	ROAD NAME US 264 MOZINGO ROAD	DAY AND TIME RESTRICTIONS 5AM - 9PM N/A	DURATION AND OPERATION PROJECT N/A
B)	REMOVE LANE CLOSURE DEVICES PERFORMED BEHIND THE LANE C	LOSURE OR WHEN A	LANE CLOSURE IS NO
C)	LONGER NEEDED OR AS DIRECTE WHEN PERSONNEL AND/OR EQUIF ADJACENT TO AN UNDIVIDED FA OPEN TRAVEL LANE, CLOSE THE STANDARD DRAWING NO. 1101.0 BARRIER OR GUARDRAIL.	MENT ARE WORKING CILITY AND WITHI E NEAREST OPEN TR	ON THE SHOULDER N 5 FT OF AN AVEL LANE USING ROADWAY
D)	WHEN PERSONNEL AND/OR EQUIP OF AN UNDIVIDED OR DIVIDED THE TRAFFIC CONTROL PLANS, BY THE ENGINEER. CONDUCT T EQUIPMENT REMAIN WITHIN THE	FACILITY, CLOSE ROADWAY STANDARD HE WORK SO THAT	THE LANE ACCORDING TO DRAWINGS, OR AS DIRECT ALL PERSONNEL AND/OR
E)	DO NOT WORK SIMULTANEOUSLY TRAVELWAY, RAMP, OR LOOP WI WITH GUARDRAIL OR BARRIER.		
PAVE	EMENT EDGE DROP OFF REQUIREME	INTS	
F)	BACKFILL AT A 6:1 SLOPE UP PAVEMENT IN AREAS ADJACENT EDGE OF PAVEMENT DROP-OFF A	TO AN OPENED TRA	
	BACKFILL DROP-OFFS THAT EXC POSTED SPEED LIMITS OF 45 M		ROADWAYS WITH
	BACKFILL DROP-OFFS THAT EXC POSTED SPEED LIMITS LESS TH		ROADWAYS WITH
	BACKFILL WITH SUITABLE COMF ENGINEER, AT NO EXPENSE TO	•	AS APPROVED BY THE
G)	DO NOT EXCEED A DIFFERENCE LANES OF TRAFFIC FOR NOMINA WARNING "UNEVEN LANES" SIGN OF EVERY HALF MILE THROUGHO	AL LIFTS OF 1.5 I IS (W8-11) IN A	NCHES. INSTALL ADVANCE DVANCE AND A MINIMUM
TRAF	FIC PATTERN ALTERATIONS		
H)	NOTIFY THE ENGINEER TWENTY TRAFFIC PATTERN ALTERATION.		R DAYS PRIOR TO ANY

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GENERAL NOTES / LOCAL NOTES

510	NING	N)	WHEN
I)	INSTALL ADVANCE WORK ZONE WARNING SIGNS WHEN WORK IS WITHIN 40 FT FROM THE EDGE OF TRAVEL LANE AND NO MORE THAN THREE (3) DAYS PRIOR TO THE BEGINNING OF CONSTRUCTION.		IN WO (MPH) OPEN STRUC
J)	ENSURE ALL NECESSARY SIGNING IS IN PLACE PRIOR TO ALTERING ANY TRAFFIC PATTERN.		DRUMS
TRAF	FIC BARRIER		
K)	INSTALL BLACK ON ORANGE ''DIP'' SIGNS (W8-2) AND/OR "BUMP" SIGNS (W8-1) 100 FT IN ADVANCE OF THE UNEVEN AREA, OR AS DIRECTED BY THE ENGINEER.	PAVI 0)	EMENT M
L)	INSTALL TEMPORARY BARRIER ACCORDING TO THE TRANSPORTATION MANAGEMENT PLANS A MAXIMUM OF TWO (2) WEEKS PRIOR TO BEGINNING WORK IN ANY LOCATION. ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION PROCEED IN A CONTINUOUS MANNER TO COMPLETE THE PROPOSED WORK IN THAT LOCATION UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS OR AS DIRECTED BY THE ENGINEER.		
	DO NOT PLACE BARRIER DIRECTLY ON ANY SURFACE OTHER THAN ASPHALT OR CONCRETE.		1)
	ONCE TEMPORARY BARRIER IS INSTALLED AT ANY LOCATION AND NO WORK IS PERFORMED BEHIND THE TEMPORARY BARRIER FOR A PERIOD LONGER THAN TWO (2) MONTHS, REMOVE / RESET TEMPORARY BARRIER AT NO		2)
	COST TO THE DEPARTMENT UNLESS OTHERWISE STATED IN THE TRANSPORTATION MANAGEMENT PLANS, TEMPORARY BARRIER IS PROTECTING A HAZARD, OR AS DIRECTED BY THE ENGINEER.		
M)	INSTALL TEMPORARY BARRIER WITH THE TRAFFIC FLOW BEGINNING WITH THE UPSTREAM SIDE OF TRAFFIC. REMOVE TEMPORARY BARRIER AGAINST THE TRAFFIC FLOW BEGINNING WITH THE DOWNSTREAM SIDE OF TRAFFIC.		
	INSTALL AND SPACE DRUMS NO GREATER THAN TWICE THE POSTED SPEED LIMIT (MPH) TO CLOSE OR KEEP THE SECTION OF THE ROADWAY CLOSED UNTIL THE TEMPORARY BARRIER CAN BE PLACED OR AFTER THE TEMPORARY BARRIER IS REMOVED.		
	PROTECT THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER AT ALL TIMES DURING THE INSTALLATION AND REMOVAL OF THE BARRIER BY EITHER A TRUCK MOUNTED ATTENUATOR (MAXIMUM 72 HOURS) OR A TEMPORARY CRASH CUSHION.		
	PROTECT THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER FROM ONCOMING TRAFFIC AT ALL TIMES BY A TEMPORARY CRASH CUSHION UNLESS THE APPROACH END OF MOVABLE/PORTABLE CONCRETE BARRIER IS OFFSET FROM ONCOMING TRAFFIC AS FOLLOWS OR AS SHOWN IN THE PLANS: (SEE ALSO 1101.05)		
	POSTED SPEED LIMIT MINIMUM OFFSET 40 OR LESS 15 FT		



PROJ. REFERENCE NO. SHEET NO.
41665.13C TMP -1B

TRAFFIC CONTROL DEVICES

LANE CLOSURES ARE NOT IN EFFECT SPACE CHANNELIZING DEVICES ORK AREAS NO GREATER IN FEET THAN TWICE THE POSTED SPEED LIMIT) EXCEPT, 10 FT ON-CENTER IN RADII, AND 3 FT OFF THE EDGE OF AN TRAVELWAY. REFER TO STANDARD SPECIFICATIONS FOR ROADS AND CTURES SECTIONS 1130 (DRUMS), 1135 (CONES) AND 1180 (SKINNY S) FOR ADDITIONAL REQUIREMENTS.

MARKINGS

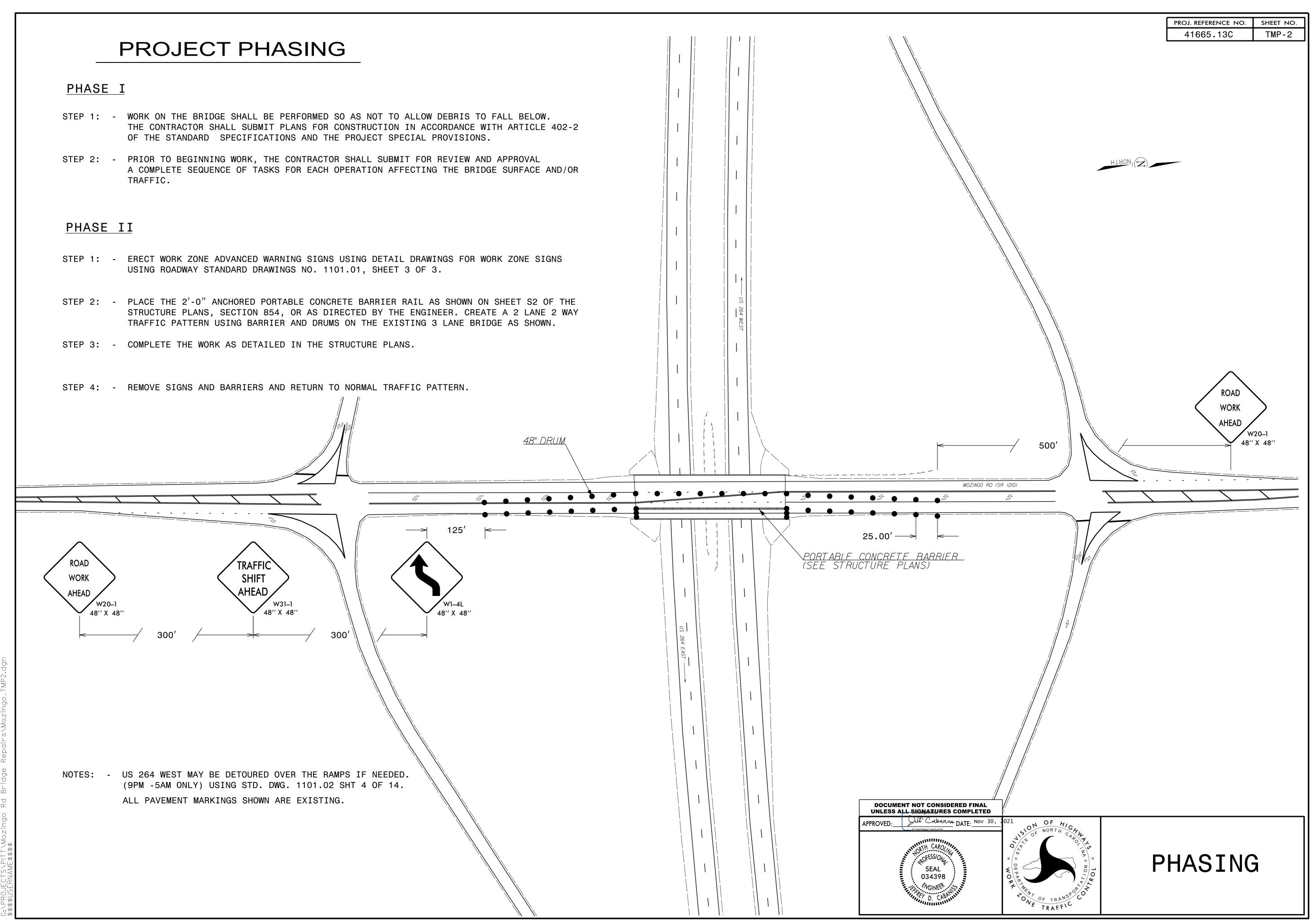
MENT MARKINGS WILL BE INSTALLED BY DOT FORCES.

LOCAL NOTES

CAL NOTES:

EMERGENCY VEHICLE ACCESS MUST BE MAINTAINED AT ALL TIMES. NOTIFY THE FIRE DEPT, E.M.S., AND PITT COUNTY SCHOOL BOARD 30 DAYS PRIOR TO ROAD CLOSURE. LOCAL ACCESS TO ALL RESIDENCES AND BUSINESSES WILL BE MAINTAINED BETWEEN CLOSURE POINTS AT ALL TIMES DURING CONSTRUCTION.

> TRANSPORTATION OPERATIONS PLAN (MANAGEMENT STRATEGIES & GENERAL NOTES)



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NOTES:

REPAIR LOCATIONS AND ESTIMATES OF QUANTITIES ARE GIVEN WITH THE BEST INFORMATION AVAILABLE. IF ADDITIONAL REPAIRS NOT SHOWN ON THE DRAWINGS ARE DEEMED NECESSARY BY THE ENGINEER, THE ENGINEER SHALL NOTE ON THE DRAWINGS THE APPROXIMATE LOCATION AND DESCRIPTION OF THE REPAIRS. EXISTING DIMENSIONS AND BRIDGE CONDITION ARE FROM THE BEST INFORMATION AVAILABLE. THE CONTRACTOR SHALL FIELD VERIFY THE INFORMATION SHOWN ON THE PLANS AND NOTIFY THE ENGINEER IF ACTUAL DIMENSIONS AND CONDITIONS DIFFER.

THE CONTRACTOR SHALL HAVE NO CLAIM WHATSOEVER AGAINST THE DEPARTMENT FOR ANY DELAYS OR ADDITIONAL COST INCURRED BASED ON DIFFERENCES BETWEEN WHAT IS SHOWN ON THE PLANS AND THE ACTUAL CONDITIONS AT THE PROJECT SITE.

IT IS THE CONTRACTOR'S RESPONSIBILITY TO FOLLOW ALL STATE AND FEDERAL SAFETY REQUIREMENTS. WORK ON THE BRIDGES SHALL BE PERFORMED SO AS NOT TO ALLOW DEBRIS TO FALL BELOW. THE CONTRACTOR SHALL SUBMIT PLANS FOR CONSTRUCTION IN ACCORDANCE WITH ARTICLE 402-2 OF THE STANDARD SPECIFICATIONS AND THE PROJECT SPECIAL PROVISIONS.

THE CONTRACTOR SHALL PERFORM ALL WORK WITH CARE SO THAT THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE WILL NOT BE DAMAGED. IF THE CONTRACTOR DAMAGES ANY PART OF THE EXISTING STRUCTURE WHICH IS TO REMAIN IN PLACE, THE DAMAGED AREA SHALL BE REPAIRED OR REPLACED IN A MANNER SATISFACTORY TO THE ENGINEER AT NO ADDITIONAL COST TO THE DEPARTMENT. THE CONTRACTOR SHALL DETERMINE EXTENT OF WORKING AREA, STAGING PROCESS, AND INSTALL COVER 🕑 ASSEMBLY AS NECESSARY TO MEET THE REQUIREMENTS OF TRAFFIC MANAGEMENT PLANS. PRIOR TO BEGINNING WORK, THE CONTRACTOR SHALL SUBMIT FOR REVIEW AND APPROVAL A COMPLETE SEQUENCE OF TASKS FOR EACH OPERATION AFFECTING THE BRIDGE SURFACE AND/OR TRAFFIC. FOR CONTROL OF TRAFFIC AND LIMITS ON PHASING OF CONSTRUCTION, SEE TRANSPORTATION MANAGEMENT PLANS. FOR EPOXY COATED REINFORCING STEEL, SEE 2018 NORTH CAROLINA STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES, SECTION 425. FOR SUBMITTAL OF WORKING DRAWINGS, SEE SPECIAL PROVISIONS.

FOR FALSEWORK AND FORMWORK, SEE SPECIAL PROVISIONS.

FOR GROUT FOR STRUCTURES, SEE SPECIAL PROVISIONS.

FOR CRANE SAFETY, SEE SPECIAL PROVISIONS.

FOR SILANE TREATMENT FOR PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS. FOR EPOXY RESIN INJECTION, SEE SPECIAL PROVISIONS.

FOR REPAIRS TO PRESTRESSED CONCRETE GIRDERS, SEE SPECIAL PROVISIONS.

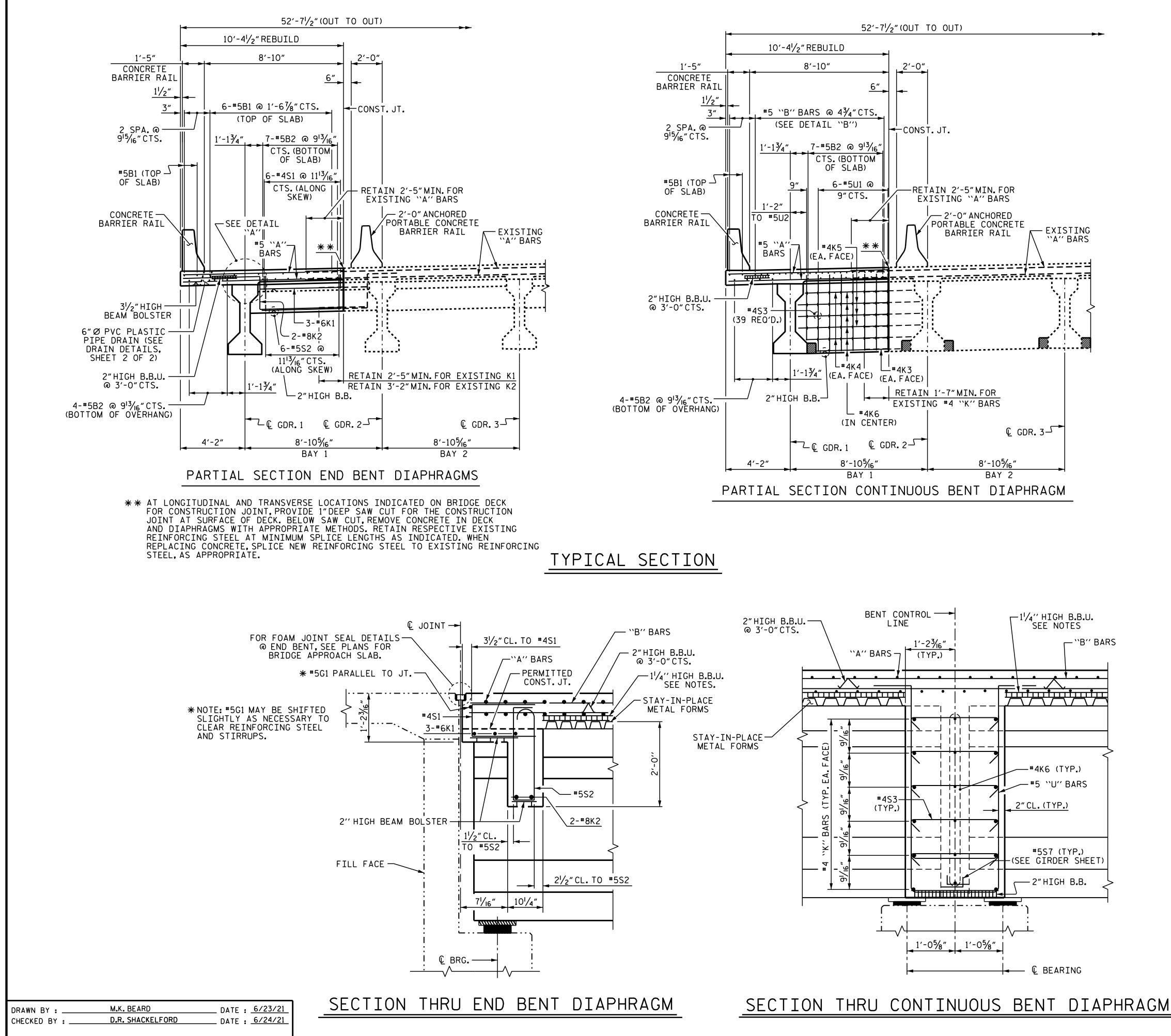
IT MAY BE DETERMINED IN THE FIELD THAT OTHER WORK WILL BE NECESSARY TO PROPERLY COMPLETE THE INTENDED BRIDGE REPAIR WORK. THE CONTRACTOR SHALL BE PREPARED TO PERFORM SUCH WORK IN A TIMELY MANNER, AS DETERMINED IN THE FIELD. SUCH WORK SHALL BE CONSIDERED EXTRA WORK AND SHALL BE ADDRESSED AS PER ARTICLE 104-7 OF THE STANDARD SPECIFICATIONS. PROJECT SPECIAL PROVISIONS THAT OUTLINE REQUIREMENTS FOR THESE POTENTIAL ADDITIONAL WORK ITEMS HAVE BEEN PROVIDED IN PROJECT DOCUMENTS, BUT NO QUANTITIES HAVE BEEN LISTED. ACTUAL PAY ITEMS, QUANTITIES, AND COSTS WILL BE ESTABLISHED, AS REQUIRED, IF EXTRA WORK IS ENCOUNTERED.

	TOTAL BILL OF MATERIAL									
	REINFORCED GROOVING AASHTO CONCRETE BRIDGE TYPE IV DECK SLAB FLOORS PRESTRESSED CONC.GIRDER				EPOXY RESIN INJECTION	PARTIAL REMOVAL OF EXISTING STRUCTURE AT BRIDGE NO. 730472	REPAIRS TO PRESTRESSED CONCRETE GIRDERS	SURFACE PREPARATION FOR PRESTRESSED CONCRETE GIRDERS	SILANE TREATMENT FOR PRESTRESSED CONCRETE GIRDERS	
	SQ.FT.	SQ.FT.	NO.	LIN.FT.	LIN.FT.	LIN.FT.	LUMP SUM	CU.FT.	SQ.YD.	SQ.YD.
SUPERSTRUCTURE	911.2	643	1	83.44	87.99	40.00	LUMP SUM	5.8	339.5	339.5
TOTAL	911.2	643	1	83.44	87.99	40.00	LUMP SUM	5.8	339.5	339.5

DRAWN BY :	M.K. BEARD	DATE :	9/21/21
CHECKED BY :	D.R. SHACKELFORD	DATE :	9/28/21

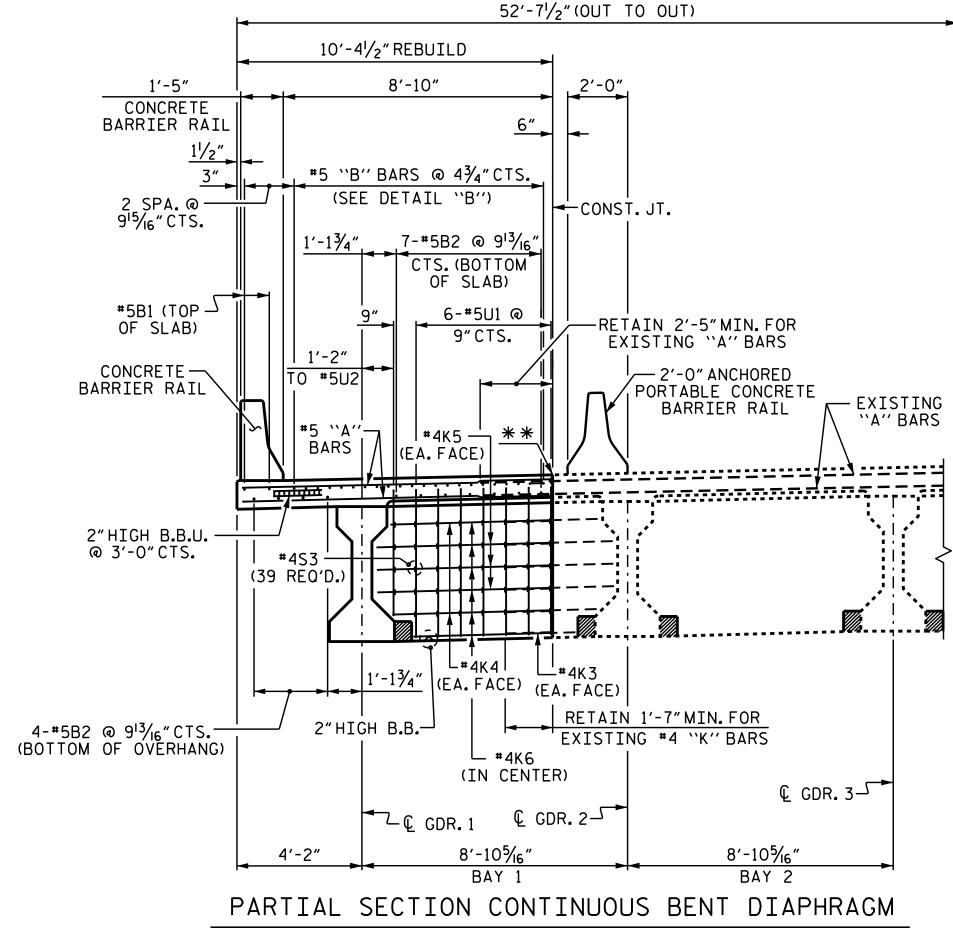
FOR PRESTRESSED CONCRETE GIRDER REPAIR DETAILS, SEE PRESTRESSED CONCRETE GIRDER REPAIRS DETAIL SHEET.

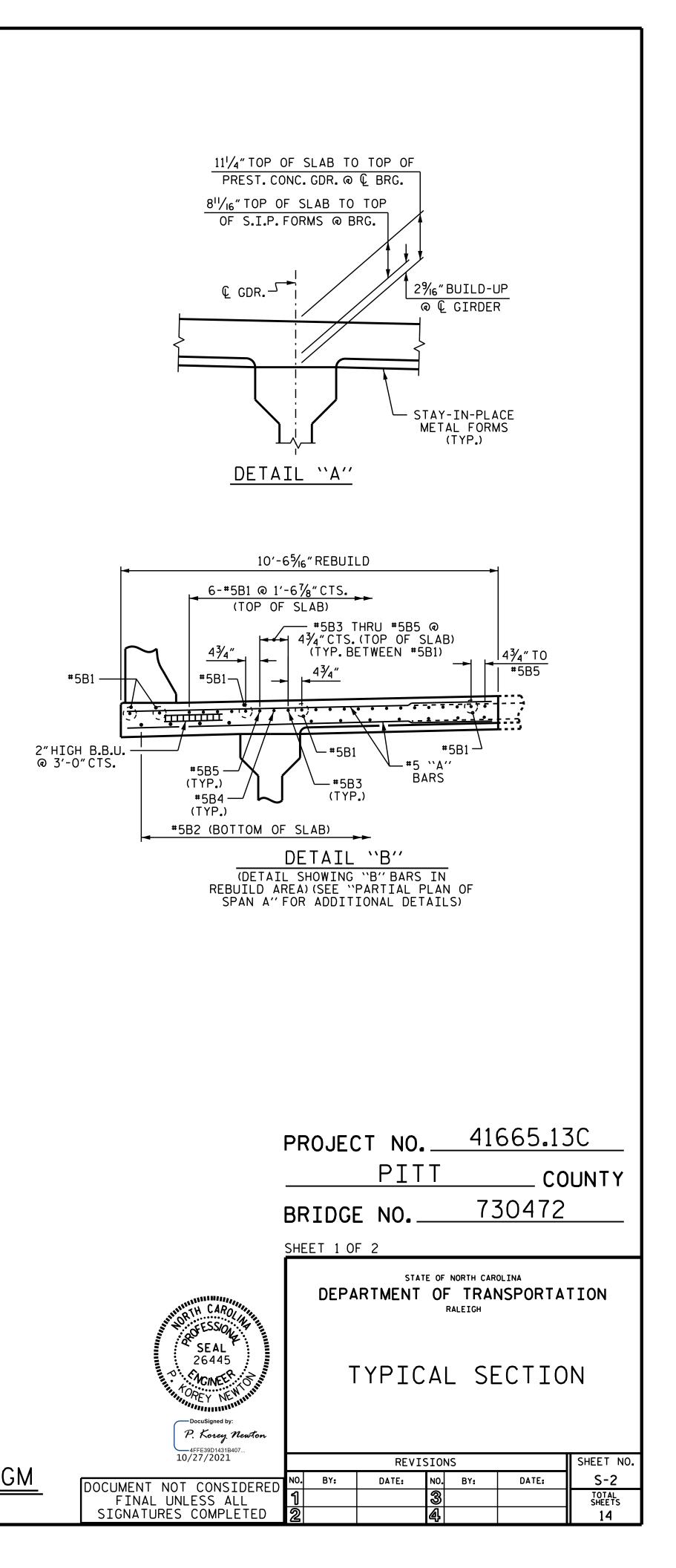
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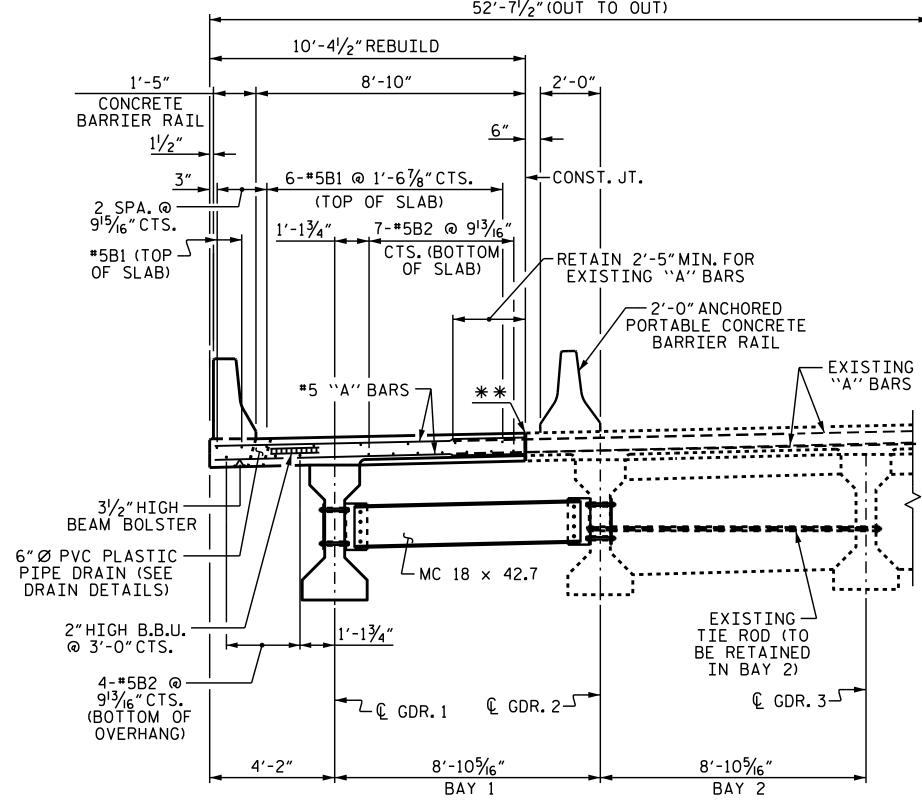


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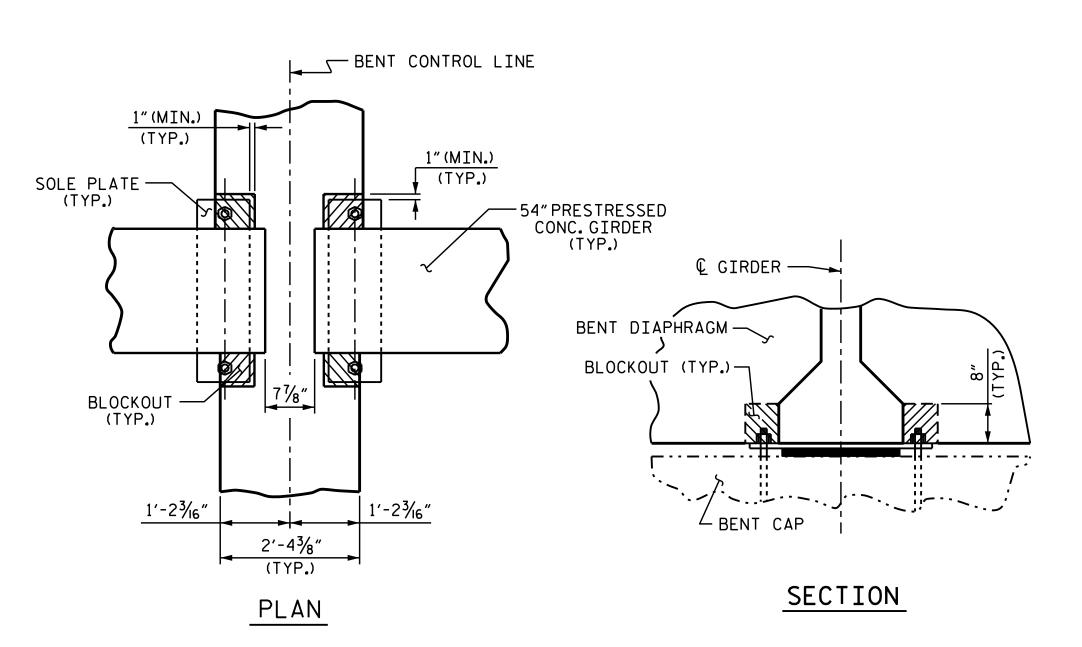
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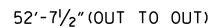
PARTIAL TYPICAL SECTION @ INTERMEDIATE DIAPHRAGM



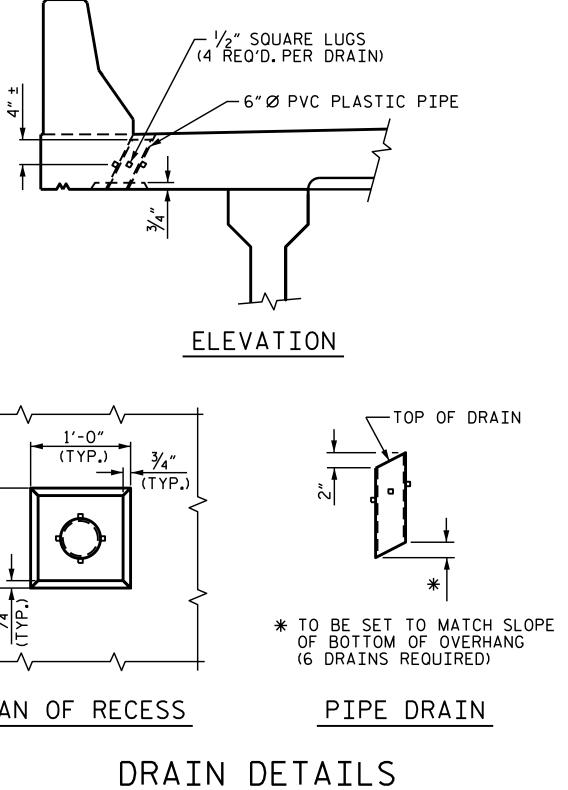
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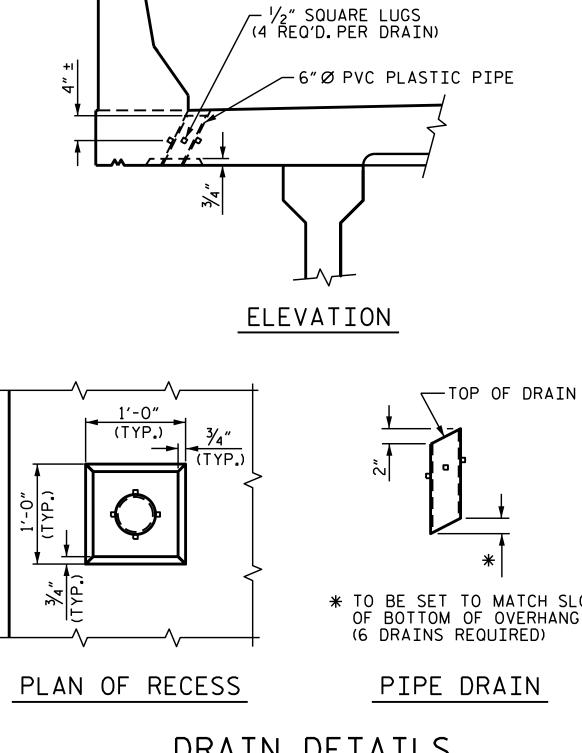
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CHECKED BY :	D.R. SHACKELFORD	DATE : 6/24/21



BENT DIAPHRAGM BLOCKOUT DETAIL

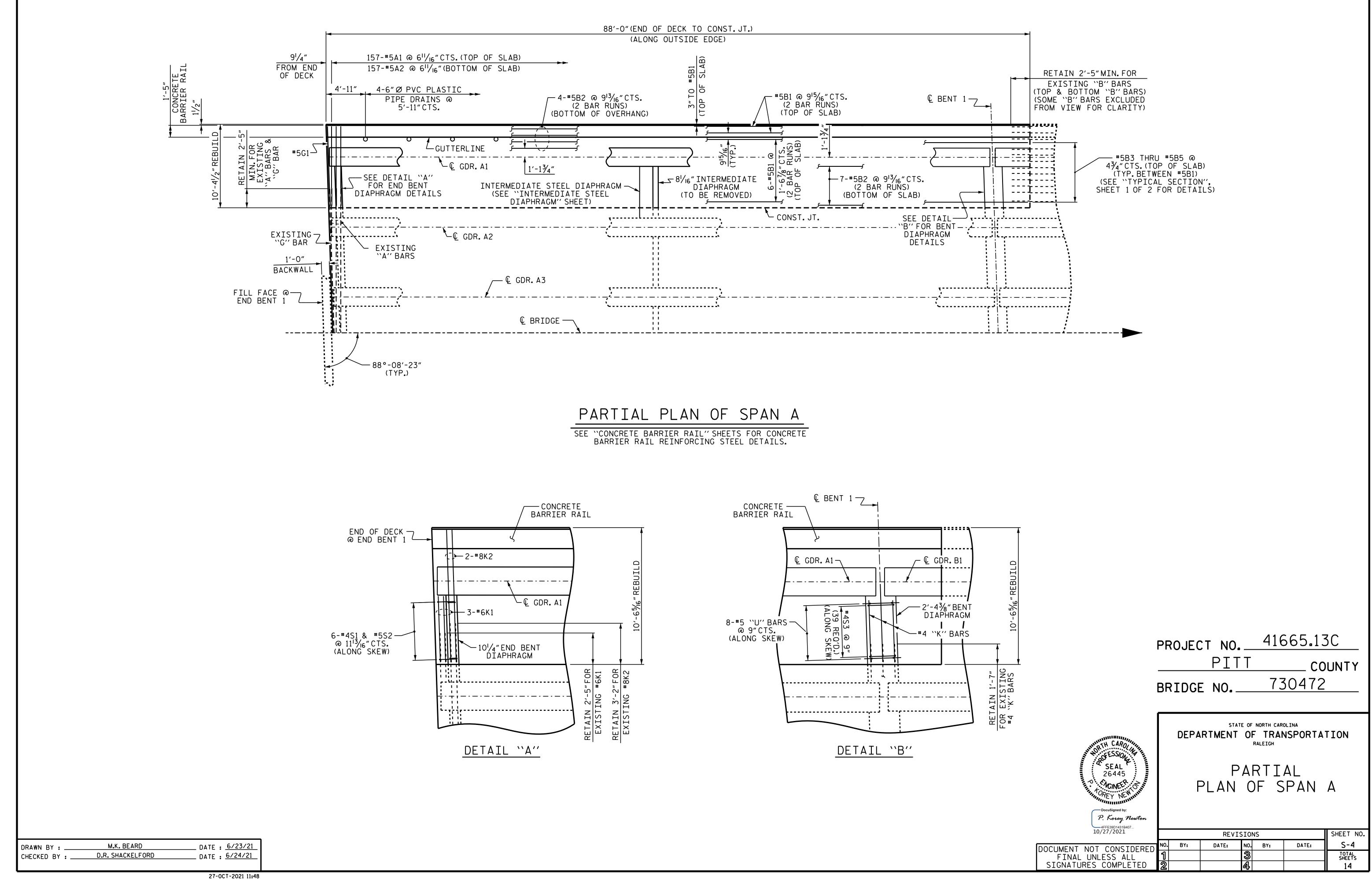


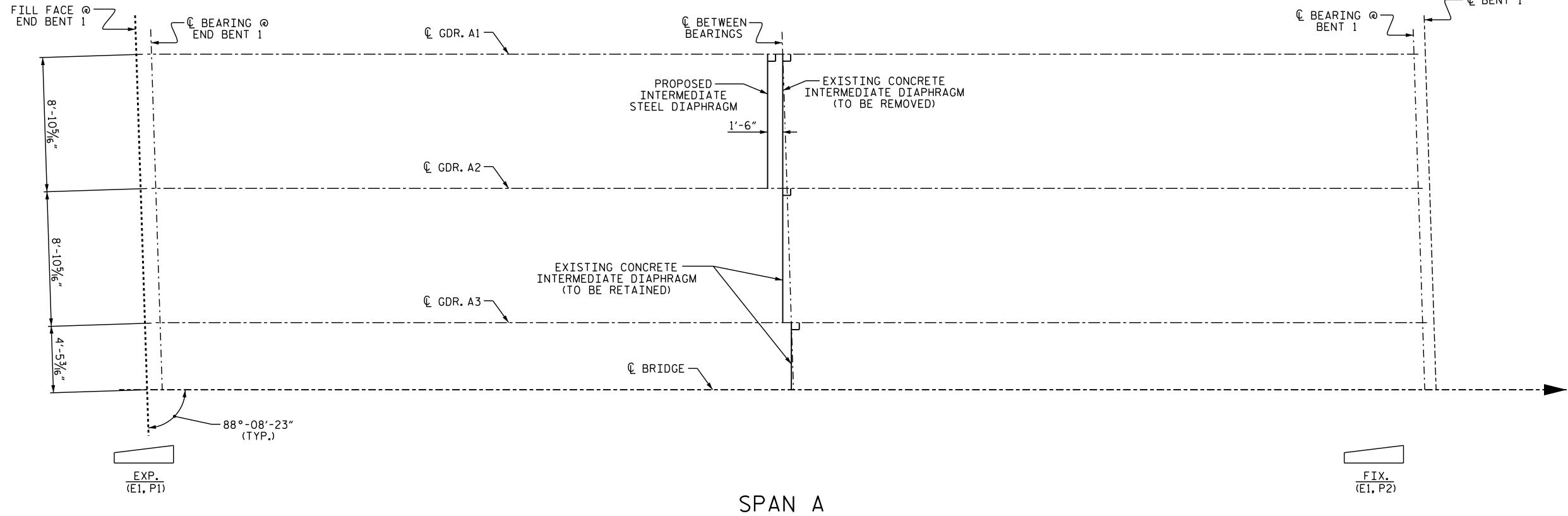


TOP OF FLOOR DRAINS TO BE SET $\frac{3}{8}$ " BELOW SURFACE OF SLAB. 4 - $\frac{1}{2}$ " SQUARE LUGS TO BE GLUED TO THE P.V.C. PLASTIC PIPE AT EQUAL SPACES AROUND THE PIPE DRAIN APPROXIMATELY 4"FROM THE TOP OF THE PIPE.

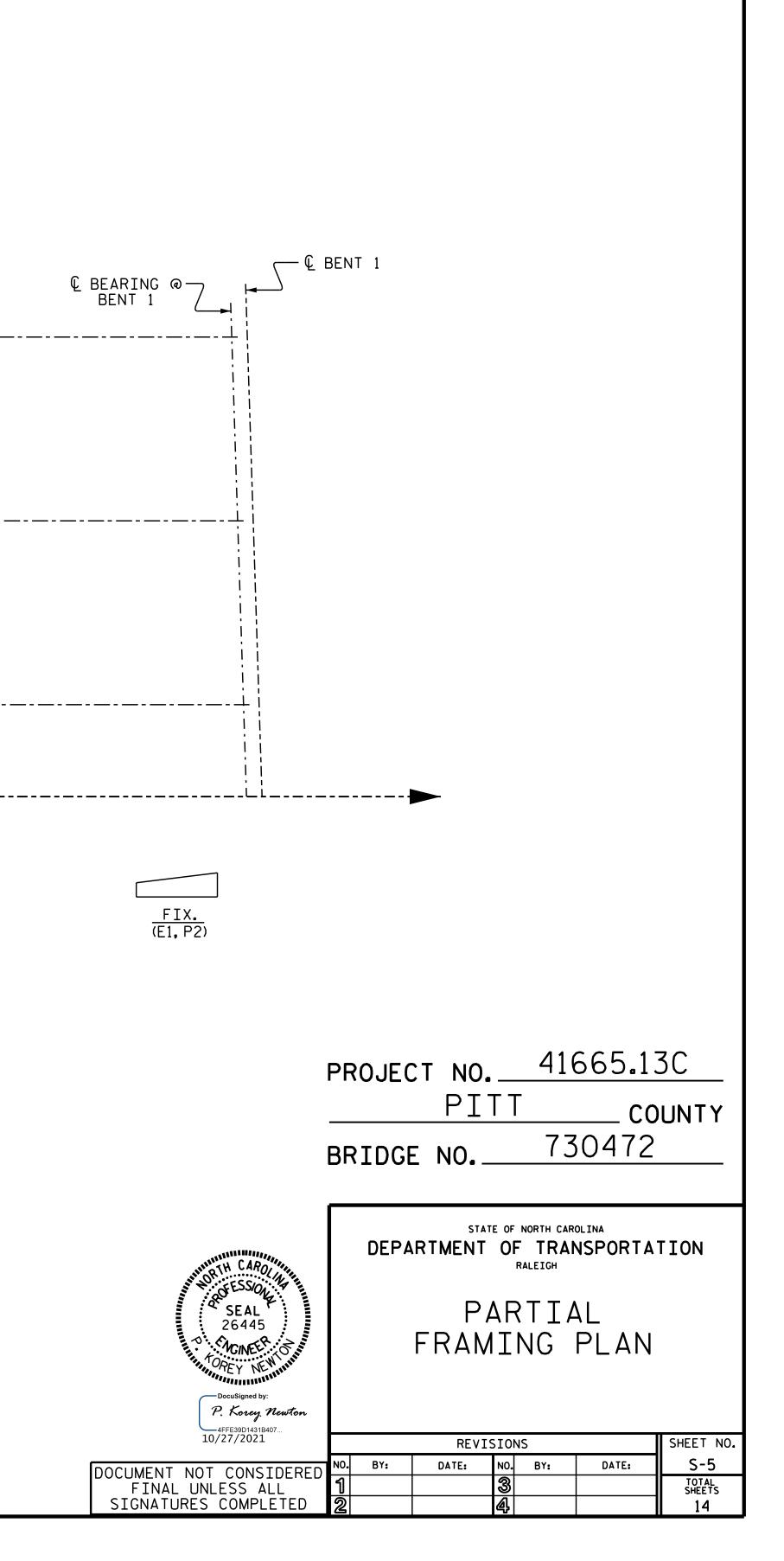
THE 6"Ø PVC PLASTIC PIPE AND FITTINGS SHALL BE SCHEDULE 40 AND CONFORM TO ASTM D1785.

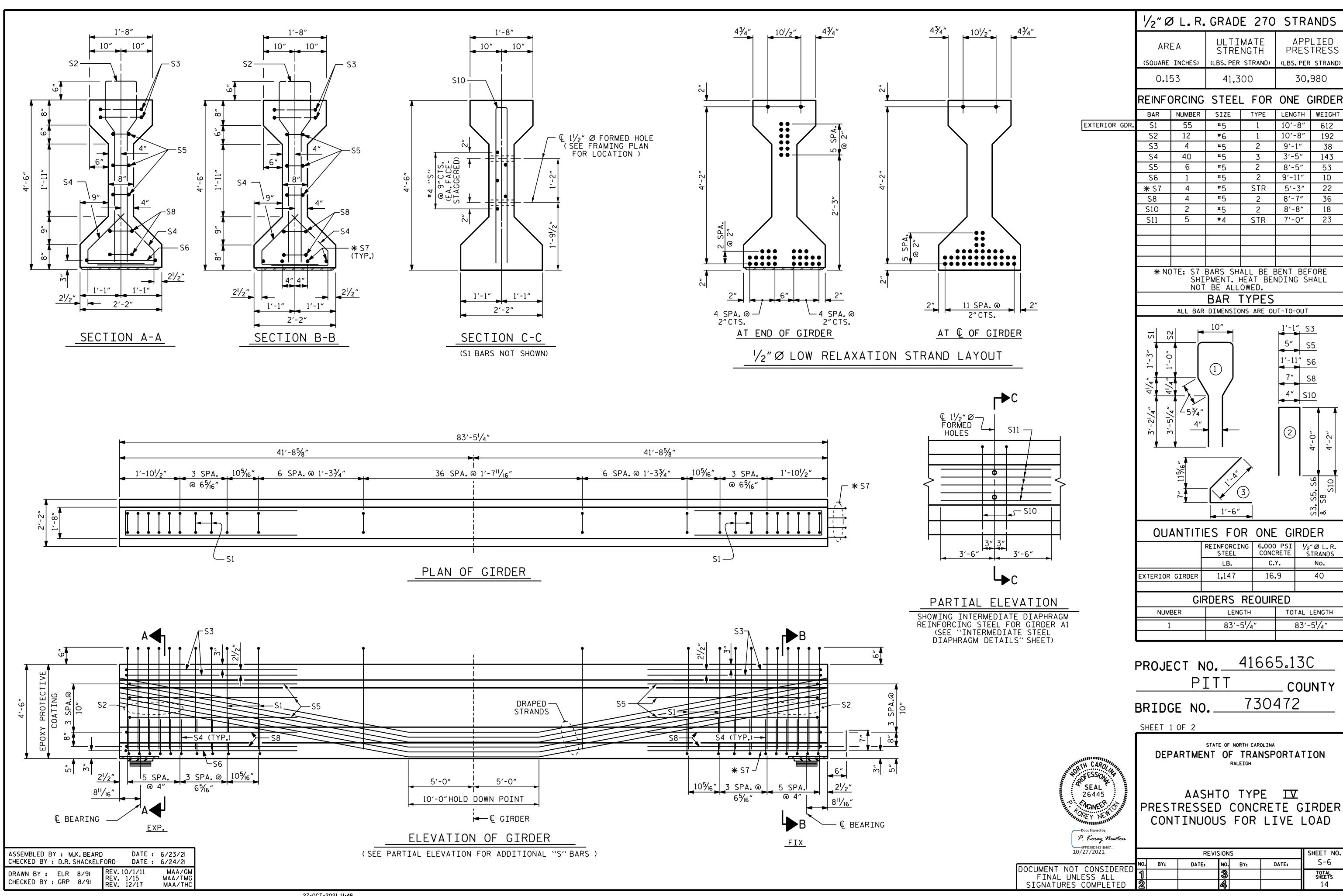
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CHECKED BY : _	D.R. SHACKELFORD	DATE : 6/24/21





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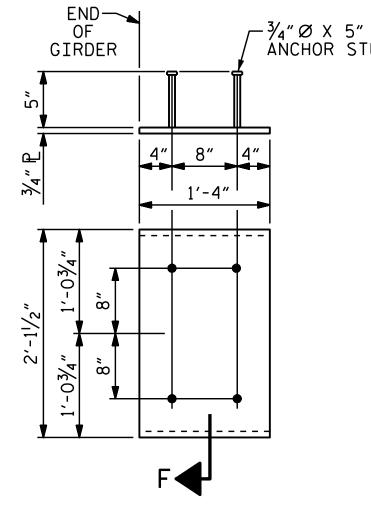
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DEAD LOAD DEFLECTION TABLE FOR GIRDER											
	SPAN A										
1/2″ØLOW RELAXATION	GIRDER 1										
TENTH POINTS	0	.1	.2	.3	.4	.5	.6	.7	.8	.9	0
CAMBER (GIRDER ALONE IN PLACE)	0	.079	.148	.203	.240	.253	.240	.203	.148	.079	0
* DEFLECTION DUE TO SUPERIMPOSED D.L.	0	.033	.059	.082	.095	.102	.095	.082	.059	.033	0
FINAL CAMBER	0	9/16″	11⁄16″	17⁄16″	1¾″	1 ¹³ ⁄16″	1¾″	17⁄16″	11⁄16″	⁹ ⁄16″	0

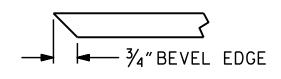
* INCLUDES FUTURE WEARING SURFACE ALL VALUES ARE SHOWN IN FEET (DECIMAL FORM), EXCEPT `` FINAL CAMBER '', WHICH IS GIVEN IN INCHES (FRACTION FORM).

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ASSEMBLED BY : M.K. BEARD CHECKED BY : D.R. SHACKELF	FORD		6/23/21 6/24/21
DRAWN BY : ELR 11/91 CHECKED BY : GRP 11/91		1/15 2/15 12/17	MAA/TMG MAA/TMG MAA/THC

ANCHOR STUDS



SECTION "F" (SEE NOTES)

ALL REINFORCING STEEL SHALL BE GRADE 60.

APPLY EPOXY PROTECTIVE COATING TO END OF GIRDER SURFACES INDICATED IN ELEVATION VIEW.

EMBEDDED PLATE ``B-1'' SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

ANCHOR STUDS SHALL CONFORM TO AASHTO M169 GRADES 1010 THROUGH 1020 OR APPROVED EQUAL, AND SHALL MEET THE TYPE ``B'' REQUIREMENTS OF SUBSECTION 7.3 OF THE ANSI/AASHTO/AWS D1.5 BRIDGE WELDING CODE.

AT ENDS OF GIRDERS TO BE EMBEDDED IN CONCRETE DIAPHRAGMS OR END WALLS, PRESTRESSING STRANDS MAY EXTEND A MAXIMUM OF 2"BEYOND THE GIRDER ENDS. OTHERWISE, PRESTRESSING STRANDS SHALL BE CUT FLUSH WITH THE GIRDER ENDS.

THE TRANSFER OF LOAD FROM THE ANCHORAGES TO THE GIRDER SHALL BE DONE WHEN CONCRETE HAS REACHED A COMPRESSIVE STRENGTH OF NOT LESS THAN 4,500 PSI.

DEPENDING ON THE TYPE OF SYSTEM USED TO SUPPORT THE DECK SLAB FORMS, PRESET ANCHORS MAY BE NECESSARY IN THE PRESTRESSED CONCRETE GIRDER.

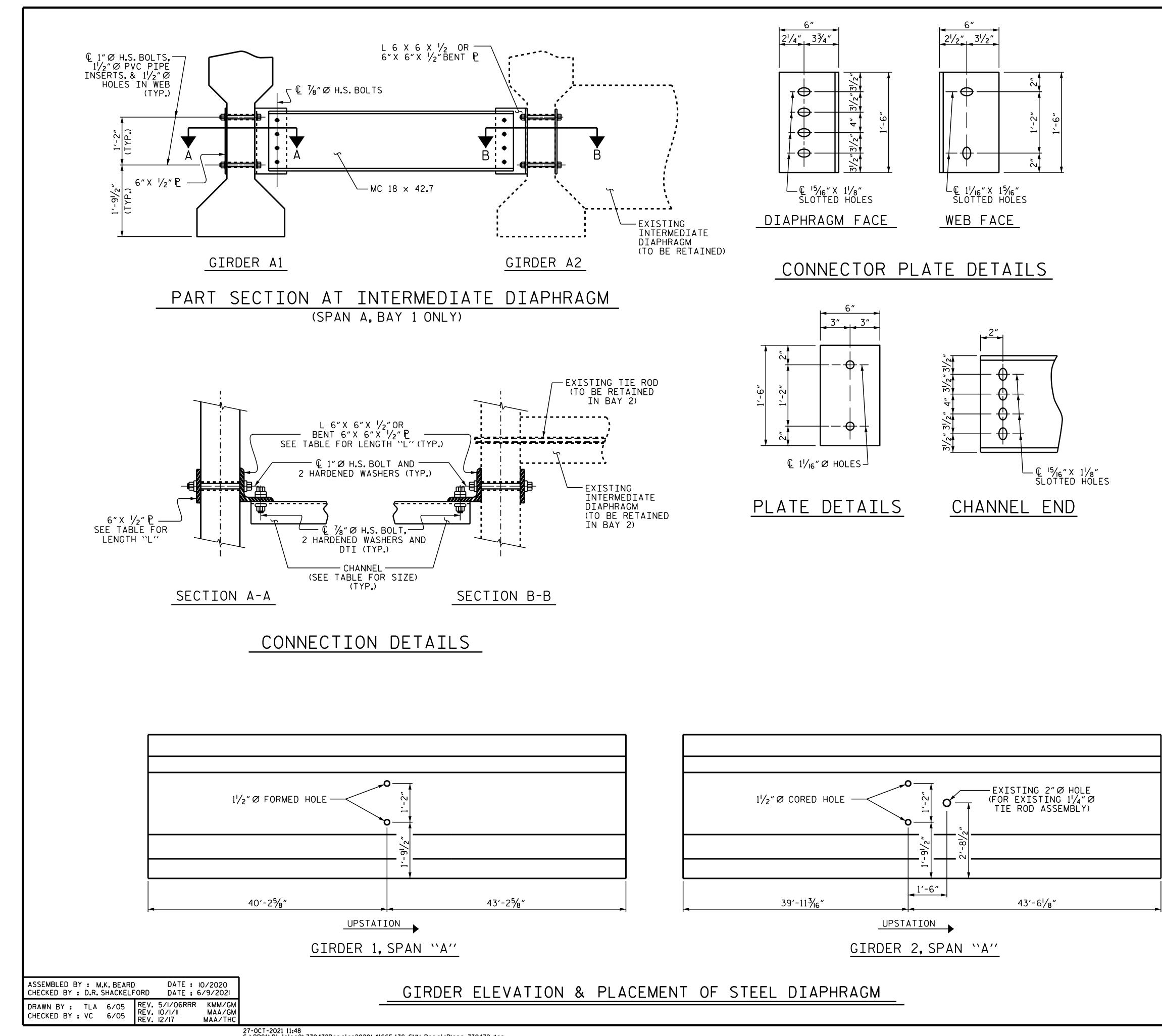
THE TOP SURFACE OF THE GIRDER, EXCLUDING THE OUTSIDE 4", SHALL BE RAKED TO A DEPTH OF 1/4".

WHEN DRAPED STRANDS ARE DETAILED, THE LONGITUDINAL LOCATION OF THE HOLD DOWN DEVICES SHALL BE WITHIN 6"OF THE LOCATION SHOWN AND THE CENTER OF GRAVITY OF THE GROUP OF DRAPED STRANDS SHALL BE LOCATED WITHIN 1/2" OF THE THEORETICAL LOCATION SHOWN.

NOTES

ALL PRESTRESSING STRANDS SHALL BE 7-WIRE LOW-RELAXATION GRADE 270 STRANDS AND SHALL CONFORM TO AASHTO M203 EXCEPT FOR SAMPLING REQUIREMENTS WHICH SHALL BE IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

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	PITTCOUNTY							
	STATION: 730472							
	SHEET 2 OF 2							
NUMBERSSION AND THE CAROL NAME	STATE OF NORTH CAROLINA DEPARTMENT OF TRANSPORTATION RALEIGH							
SEAL 26445 DocuSigned by: P. Korey. Newton 4FFE39D1431B407	PRESTRESSED CONCRETE GIRDER CONTINUOUS FOR LIVE LOAD DETAILS							
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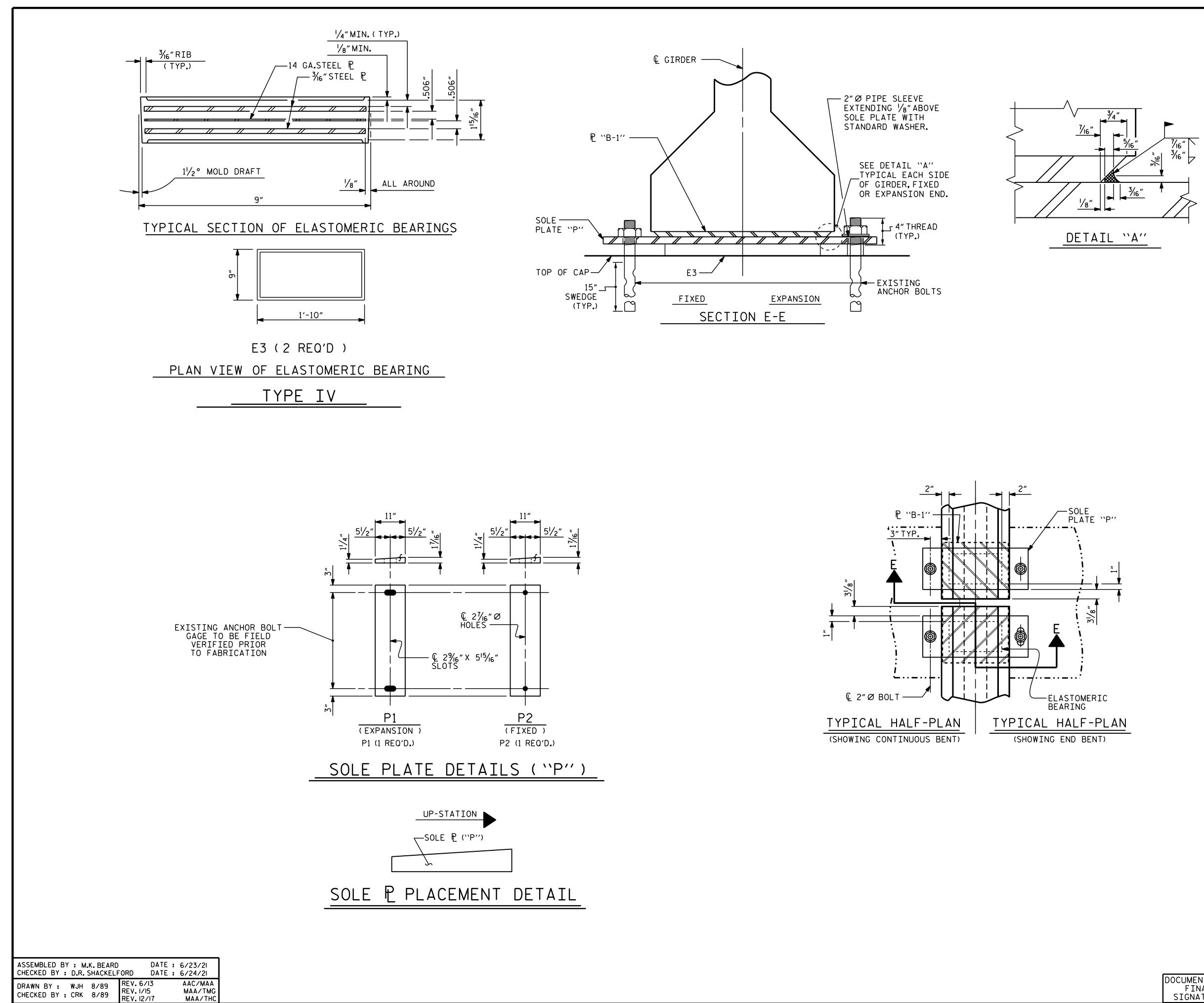
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ALL INTERMEDIATE DIAPHRAGM STEEL AND CONNECTOR PLATES SHALL BE AASHTO M270 GRADE 50 OR APPROVED EQUAL. TENSION ON THE ASTM A325 BOLTS THROUGH THE CHANNEL MEMBER SHALL BE CALIBRATED USING DIRECT TENSION INDICATOR WASHERS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. TENSION ON THE ASTM A449 BOLTS THROUGH THE GIRDER WEB SHALL BE SNUG TIGHTENED FOLLOWED BY AN ADDITIONAL $\frac{1}{4}$ TURN. THE PLATES, BENT PLATES, CHANNELS, AND ANGLES SHALL BE GALVANIZED OR METALLIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. FOR THERMAL SPRAYED COATINGS (METALLIZATION), SEE SPECIAL PROVISIONS. FOR METALLIZATION, APPLY A THERMAL SPRAYED COATING WITH A SEAL COAT TO ALL STEEL DIAPHRAGM SURFACES IN ACCORDANCE WITH THE DEPARTMENTS THERMAL SPRAYED COATINGS (METALLIZATION) PROGRAM, THERMAL SPRAYED COATINGS SPECIAL PROVISION AND SECTION 442 OF THE STANDARD SPECIFICATIONS. GALVANIZE THE HIGH STRENGTH BOLTS, NUTS, WASHERS AND DIRECT TENSION INDICATORS IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS. USE AN ASTM F436 HARDENED WASHER WITH STANDARD AND SLOTTED HOLES UNDER EACH BOLT HEAD AND NUT. FOR BOLTS THROUGH THE GIRDER WEB, PROVIDE SUFFICIENT LENGTH OF THREADS ON ALL BOLTS TO ACCOMMODATE, WASHERS AND THE THICKNESS OF CONNECTING MEMBER PLUS AT LEAST $\frac{1}{4}$ PROJECTION BEYOND THE NUT. INTERMEDIATE DIAPHRAGM ASSEMBLY SHALL COMPLY WITH SECTION 1072 OF THE STANDARD SPECIFICATIONS. SUBMIT TWO SETS OF WORKING DRAWINGS FOR THE INTERMEDIATE DIAPHRAGM ASSEMBLY FOR REVIEW, COMMENTS AND ACCEPTANCE. AFTER REVIEW, COMMENTS, AND ACCEPTANCE, SUBMIT SEVEN SETS FOR DISTRIBUTION. IN THE EXTERIOR BAYS, PLACE TEMPORARY STRUTS BETWEEN PRESTRESSED GIRDERS ADJACENT TO THE STEEL DIAPHRAGMS. STRUTS SHALL REMAIN IN PLACE 3 DAYS AFTER CONCRETE IS PLACED. THE COST OF THE STEEL DIAPHRAGMS AND ASSEMBLIES SHALL BE INCLUDED IN THE UNIT PRICE BID FOR PRESTRESSED CONCRETE GIRDERS. BOLT THROUGH GIRDER WEB HARDENED WASHER NUT (TURNED ELEMENT)------HARDENED WASHER BOLT WITH DTI ASSEMBLY DETAIL PROJECT NO. 41665.13C PITT _ COUNTY BRIDGE NO. 730472

STRUCTURAL STEEL NOTES

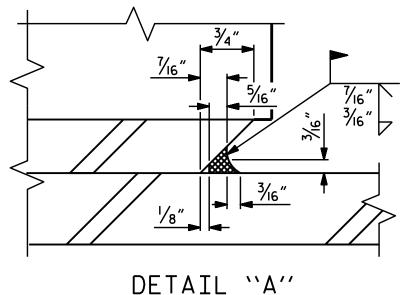
SEAL 26445 Docusigned by: P. Korey. Newton 4FFE39D1431B407	DEPARTMENT OF TRANSPORTATION RALEIGH STANDARD INTERMEDIATE STEEL DIAPHRAGMS FOR TYPE IV PRESTRESSED CONCRETE GIRDERS							
10/27/2021	REVISIONS SHEET NO.							
DOCUMENT NOT CONSIDERED	NO. B	BY:	DATE:	NO.	BY:	DATE:	S-8	
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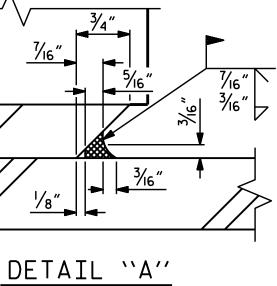


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AT ALL FIXED POINTS OF SUPPORT, NUTS FOR ANCHOR BOLTS ARE TO BE TIGHTENED FINGER TIGHT AND THEN BACKED OFF $\frac{1}{2}$ TURN. THE THREAD OF THE NUT AND BOLT SHALL THEN BE BURRED WITH A SHARP POINTED TOOL.

THE 2" Ø PIPE SLEEVE SHALL BE CUT FROM SCHEDULE 40 PVC PLASTIC PIPE. THE PVC PLASTIC PIPE SHALL MEET THE REQUIREMENTS OF ASTM D1785.

STEEL SOLE PLATES, ANCHOR BOLTS, NUTS, AND WASHERS SHALL BE GALVANIZED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

PRIOR TO WELDING, GRIND THE GALVANIZED SURFACE OF THE PORTION OF THE EMBEDDED PLATE AND SOLE PLATE THAT ARE TO BE WELDED. AFTER WELDING, DAMAGED GALVANIZED SURFACES SHALL BE REPAIRED IN ACCORDANCE WITH THE STANDARD SPECIFICATIONS.

WHEN WELDING THE SOLE PLATE TO THE EMBEDDED PLATE IN THE GIRDER, USE TEMPERATURE INDICATING WAX PENS, OR OTHER SUITABLE MEANS, TO ENSURE THAT THE TEMPERATURE OF THE SOLE PLATE DOES NOT EXCEED 300°F. TEMPERATURES ABOVE THIS MAY DAMAGE THE ELASTOMER.

SOLE PLATE "P", BOLTS, NUTS, WASHERS, AND PIPE SLEEVE SHALL BE INCLUDED IN THE PAY ITEM FOR PRESTRESSED CONCRETE GIRDERS.

ANCHOR BOLTS SHALL MEET THE REQUIREMENTS OF ASTM A449. NUTS SHALL MEET THE REQUIREMENTS OF AASHTO M291-DH OR AASHTO M292-2H. WASHERS SHALL MEET THE REQUIREMENTS OF AASHTO M293. SHOP DRAWINGS ARE NOT REQUIRED FOR ANCHOR BOLT, NUTS AND WASHERS. SHOP INSPECTION IS REQUIRED.

ALL SURFACES OF BEARING PLATES SHALL BE SMOOTH AND STRAIGHT.

FOR STEEL REINFORCED ELASTOMERIC BEARINGS, SEE SPECIAL PROVISIONS.

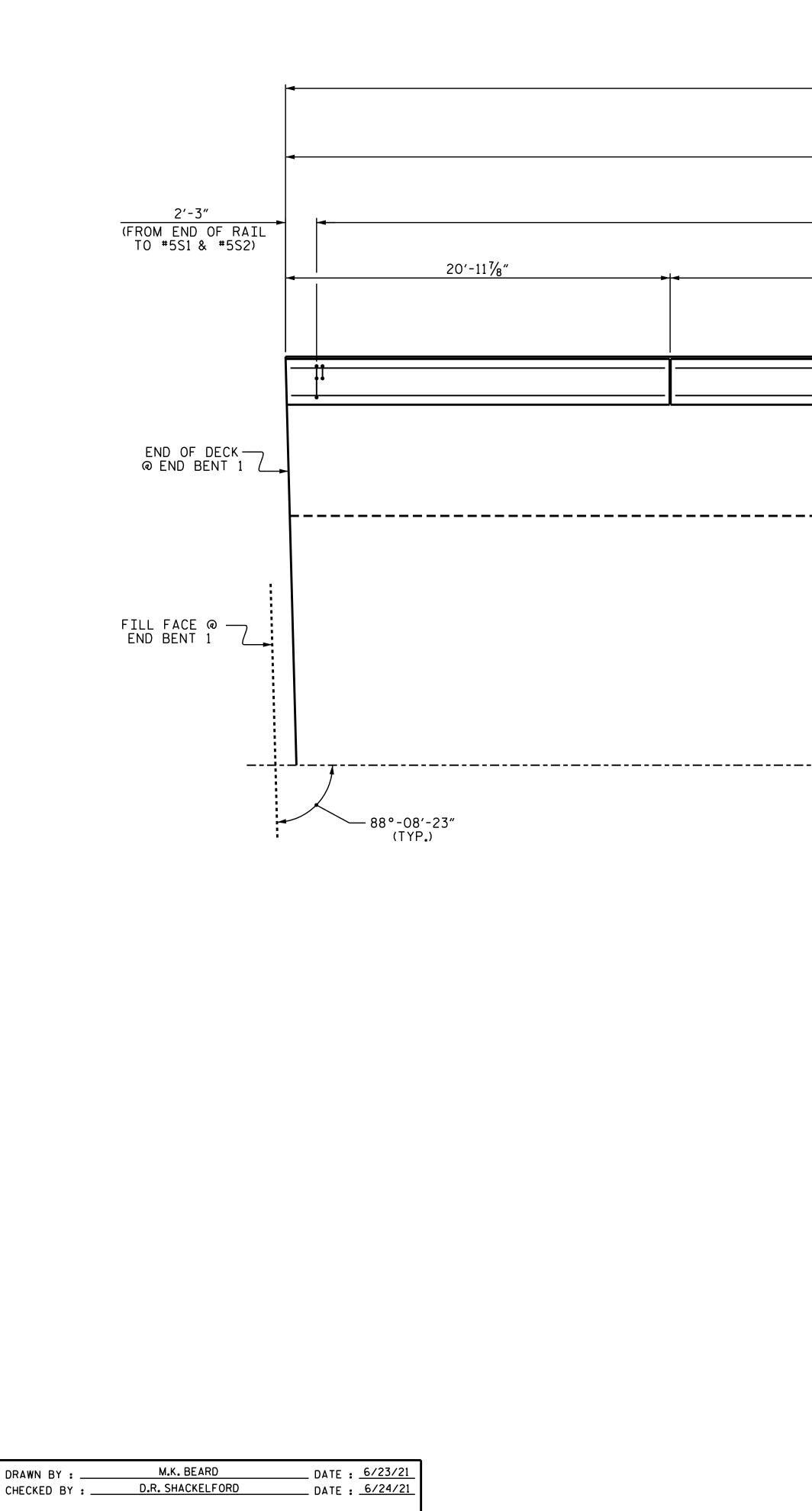
ALL SOLE PLATES SHALL BE AASHTO M270 GRADE 36.

ANCHOR BOLTS, NUTS, WASHERS AND PVC PIPES ARE TO BE RETAINED FROM EXISTING BEARING ASSEMBLIES. PLEASE TAKE CARE TO NOT DAMAGE EXISTING BOLTS, NUTS, WASHERS AND PVC PIPES.

MAXIMUM ALLOWABLE SERVICE LOADS					
D.L.+L.L.(NO IMPACT)					
TYPE IV	225 k				

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WRTH CAROLANT	DEPAI	STA RTMENT				TION
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P. Korey Newton 4FFE39D1431B407 10/27/2021				IRU	CTURE	SHEET NO.
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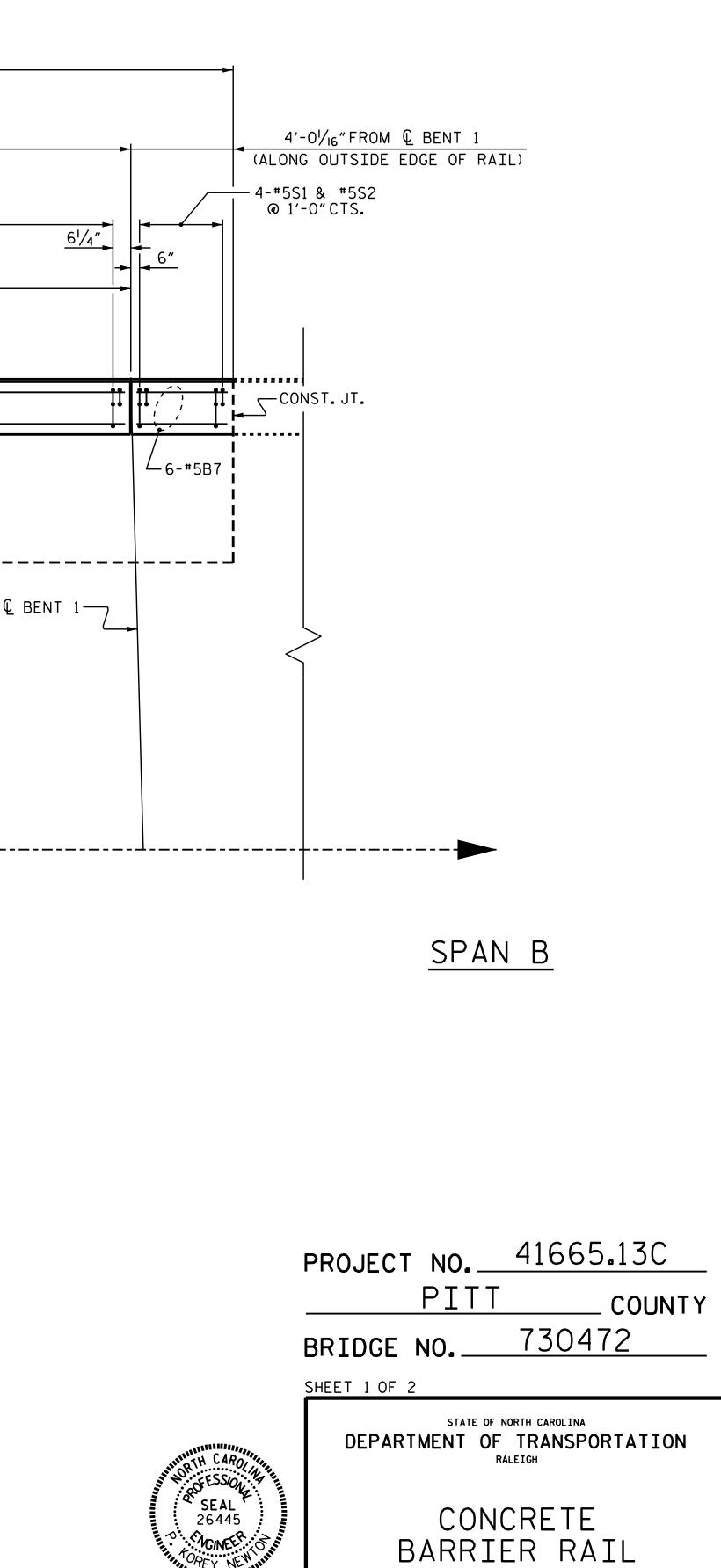
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87′	-11 ¹⁵ /16"(END OF RAIL TO CON			
	(ALONG OUTSIDE EDGE OF RA	AIL)		
83'-117/6"	(END OF RAIL TO (BENT 1))		
	G OUTSIDE EDGE OF RAIL)			
86	5-#5S1 & #5S2 @ 11½″CTS.			
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6-#5B6 (TYP.EA.	•			
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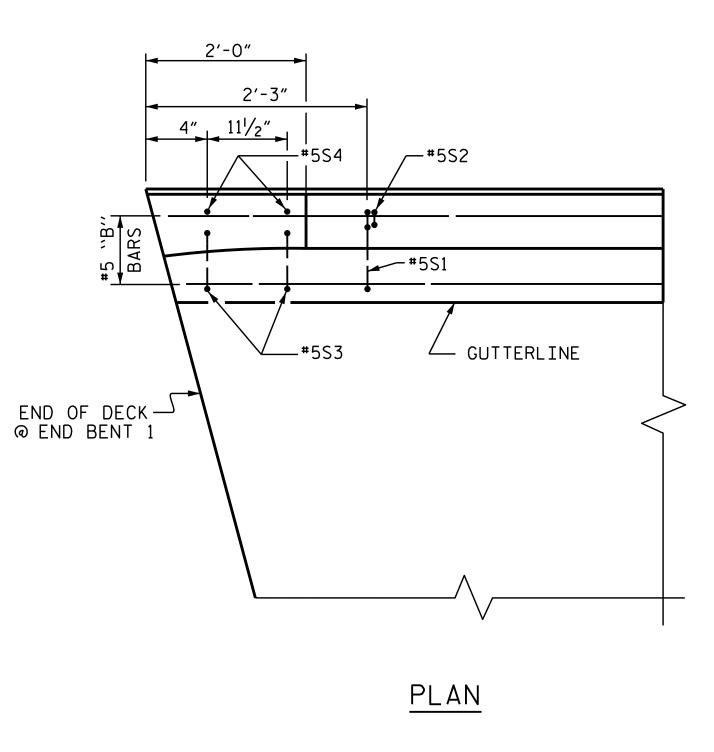
SPAN A

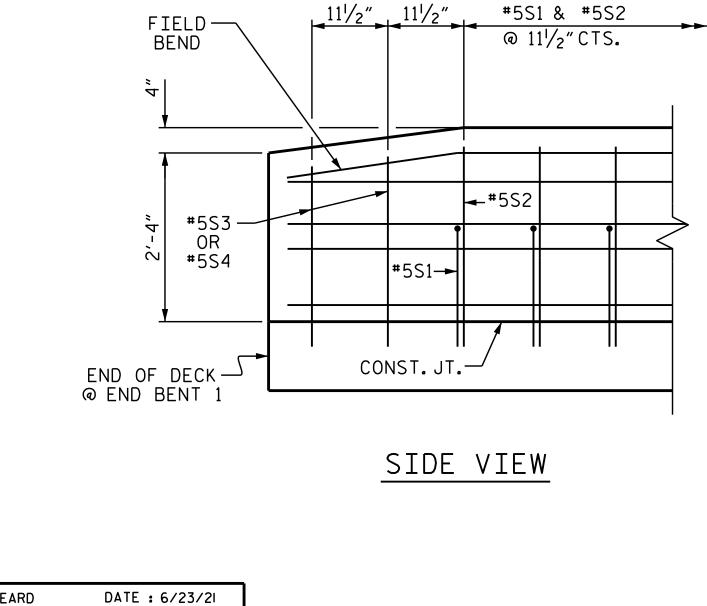
PLAN OF BARRIER RAIL

(FOR END OF RAIL REINFORCING STEEL & DETAILS, SEE SHEET 2 OF 2)



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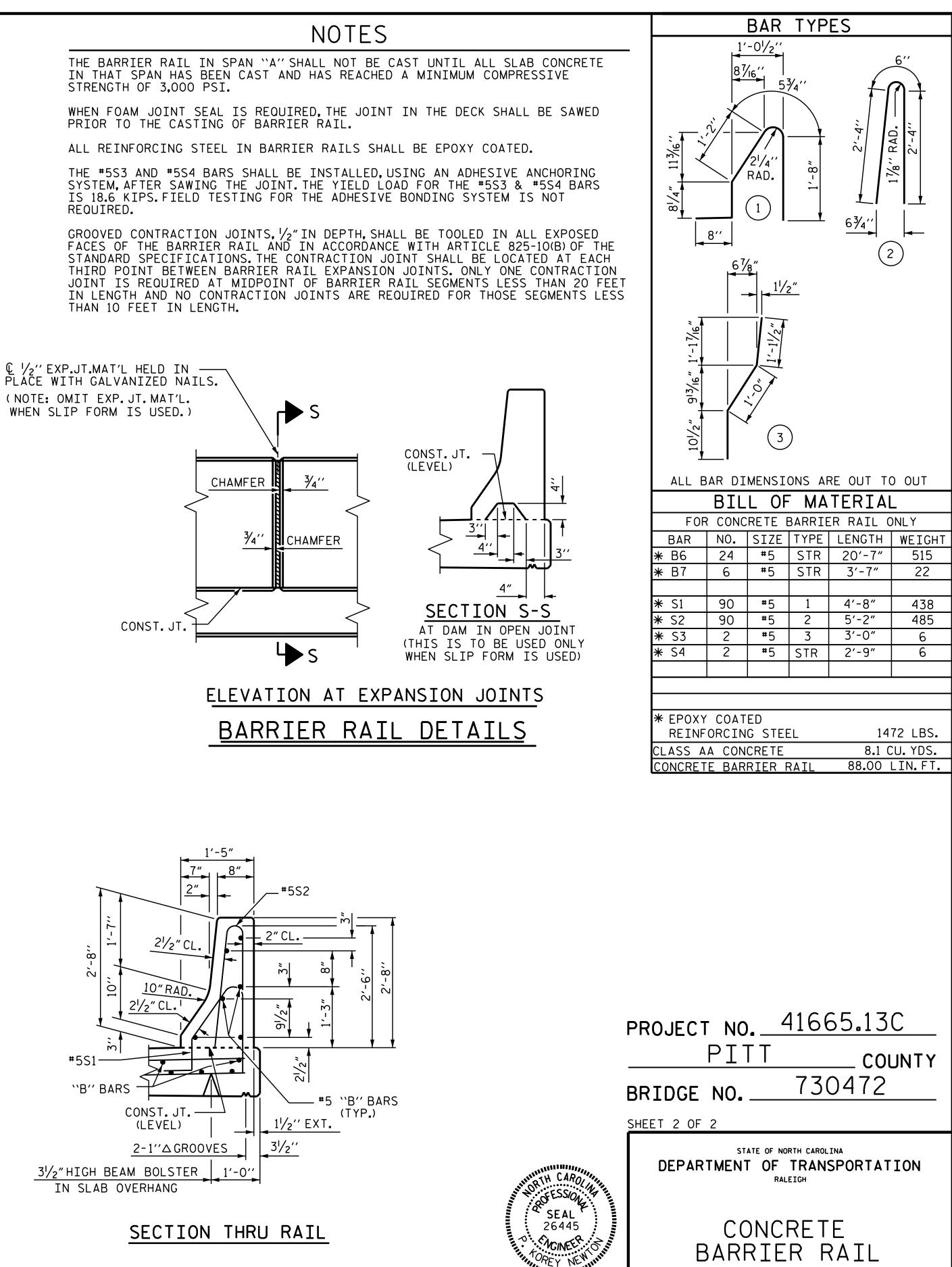


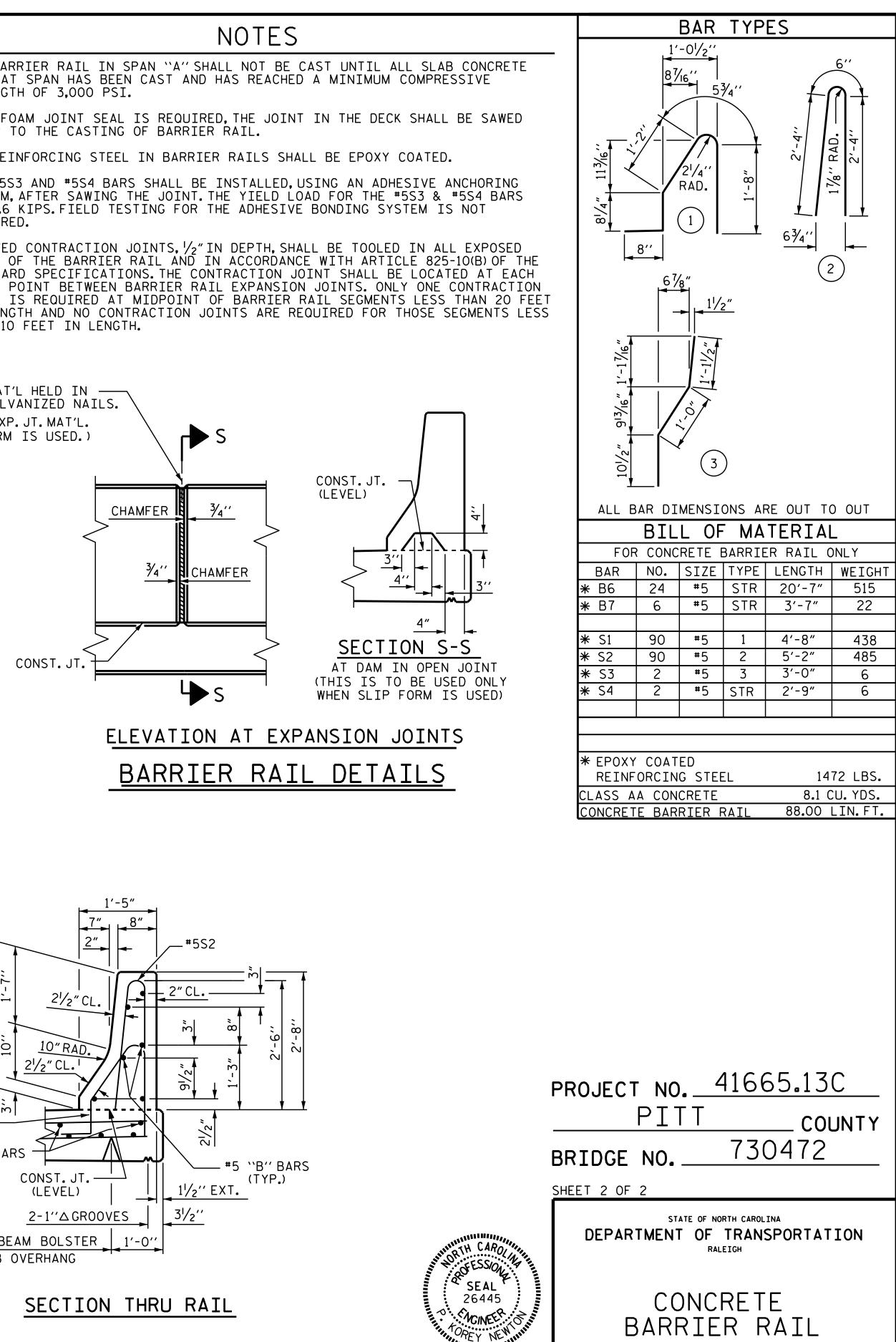


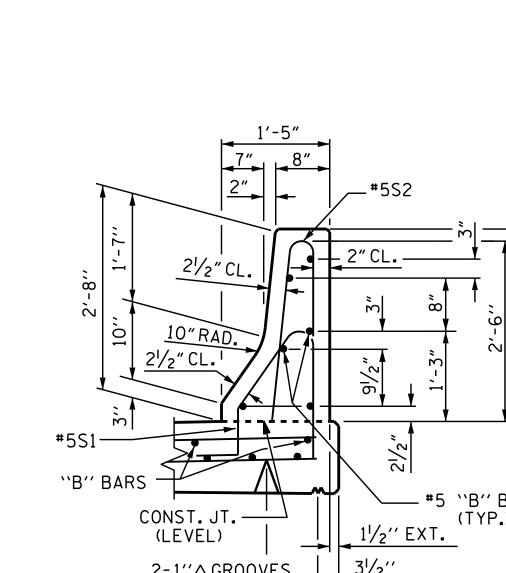
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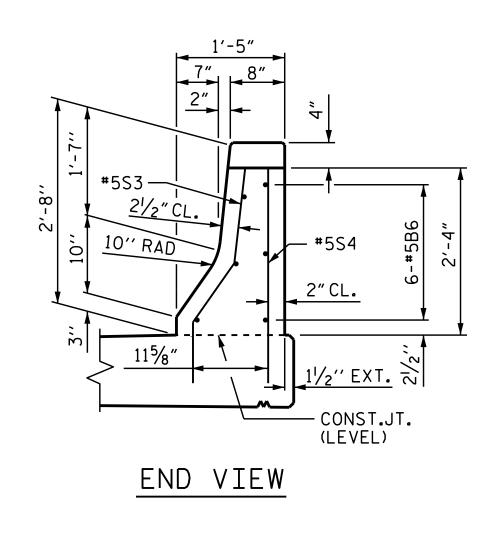
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ASSEMBLED BY : M.K. BEARD CHECKED BY : D.R. SHACKELFORD DATE : 6/23/21 DATE : 6/24/21 MAA/GM MAA/GM MAA/THC REV. 7/12 REV. 6/13 REV. 12/17 DRAWN BY : ARB 5/87 CHECKED BY : SJD 9/87









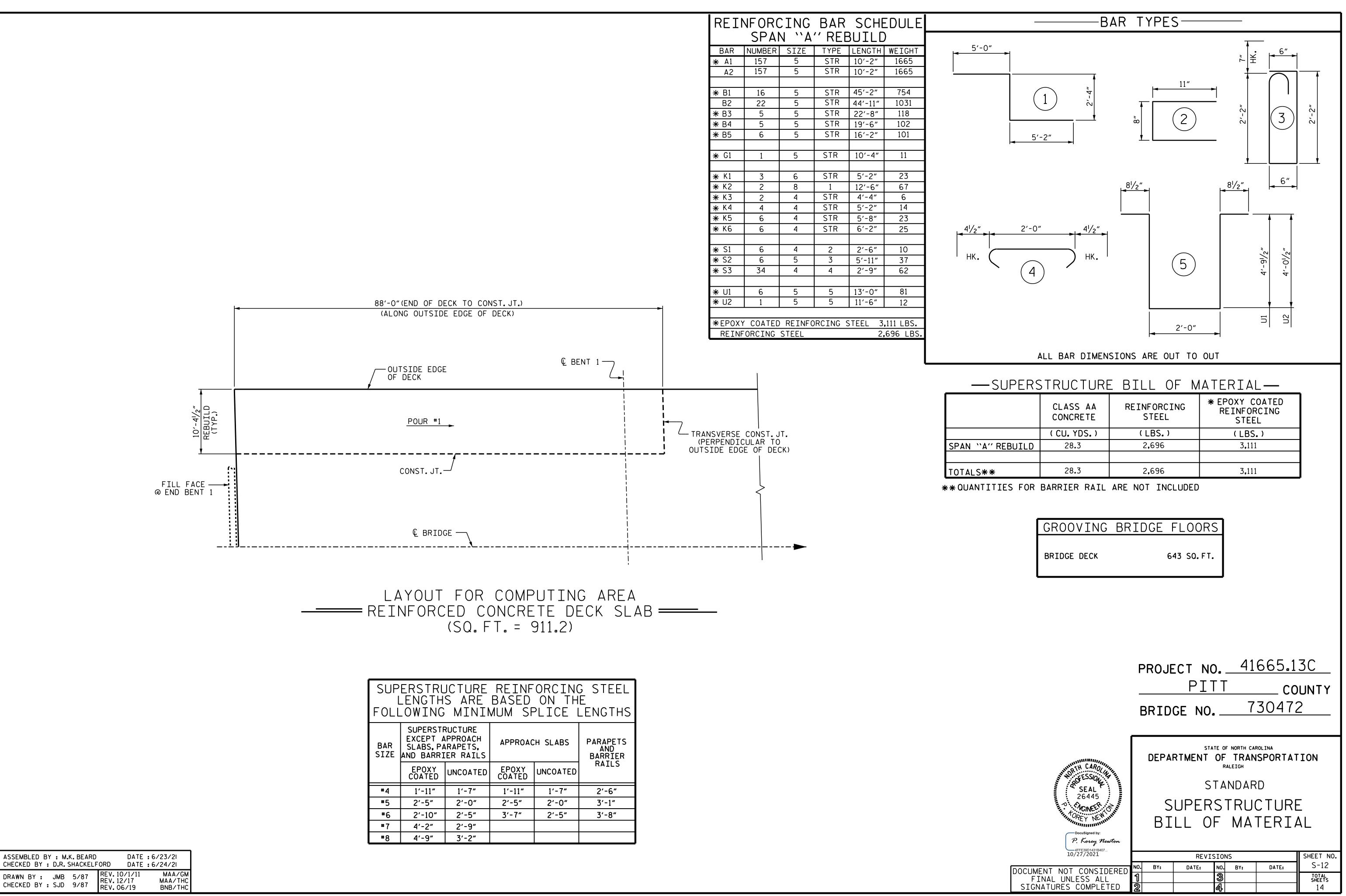
END OF RAIL DETAILS

FOR ADHESIVE ANCHORING AT SAWED JOINTS

P. Korey Newton 4FFE39D1431B407							
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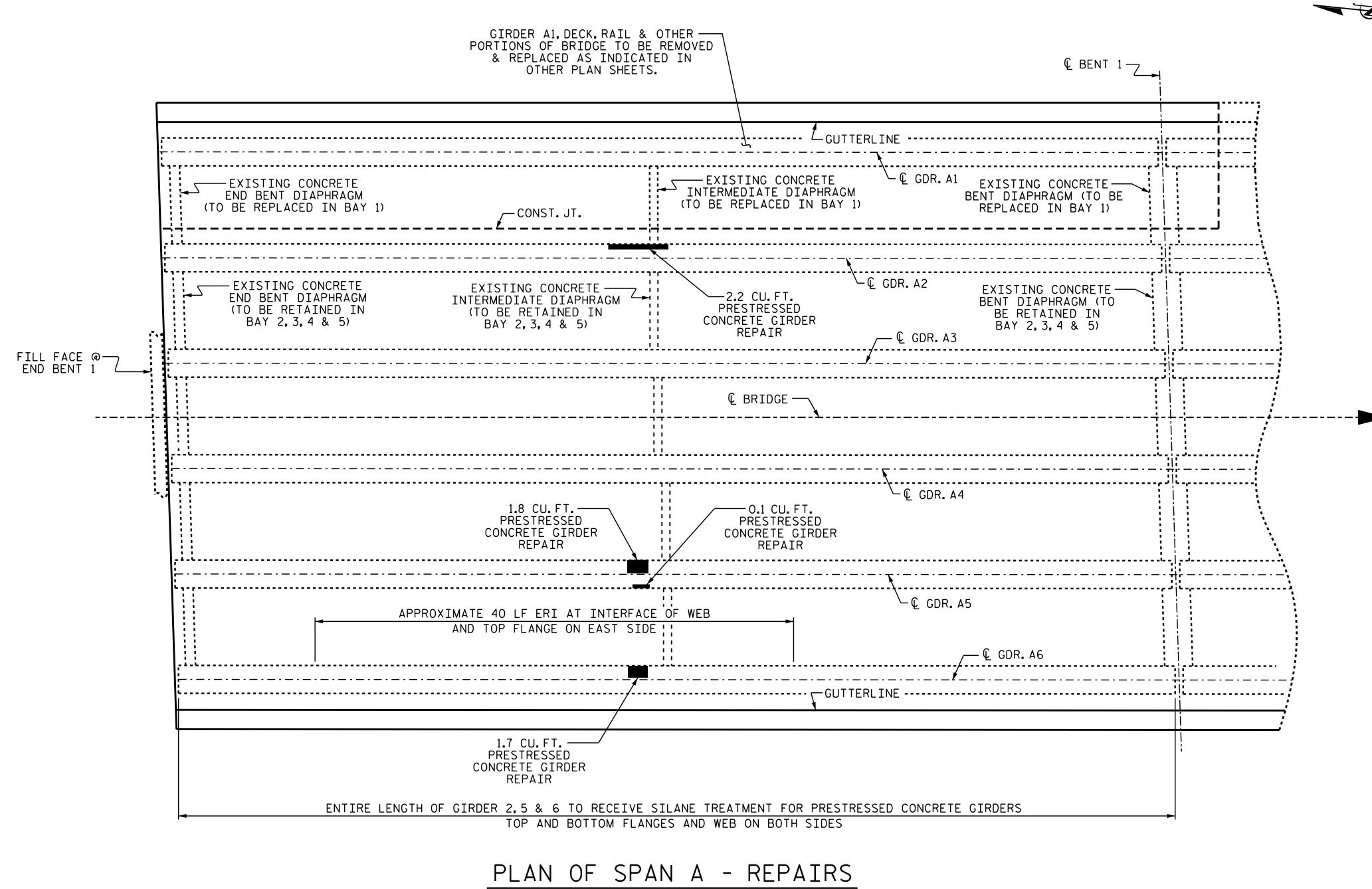
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TURE REINFORCING STEEL ARE BASED ON THE MINIMUM SPLICE LENGTHS					
TURE ROACH PETS, RAILS	APPROAC	PARAPETS AND BARRIER			
ICOATED	EPOXY COATED	UNCOATED	RAILS		
1'-7"	1'-11"	1'-7"	2'-6"		
2'-0"	2′-5″	2'-0"	3'-1"		
2'-5"	3'-7"	2'-5″	3'-8"		
2'-9"					
3'-2″					

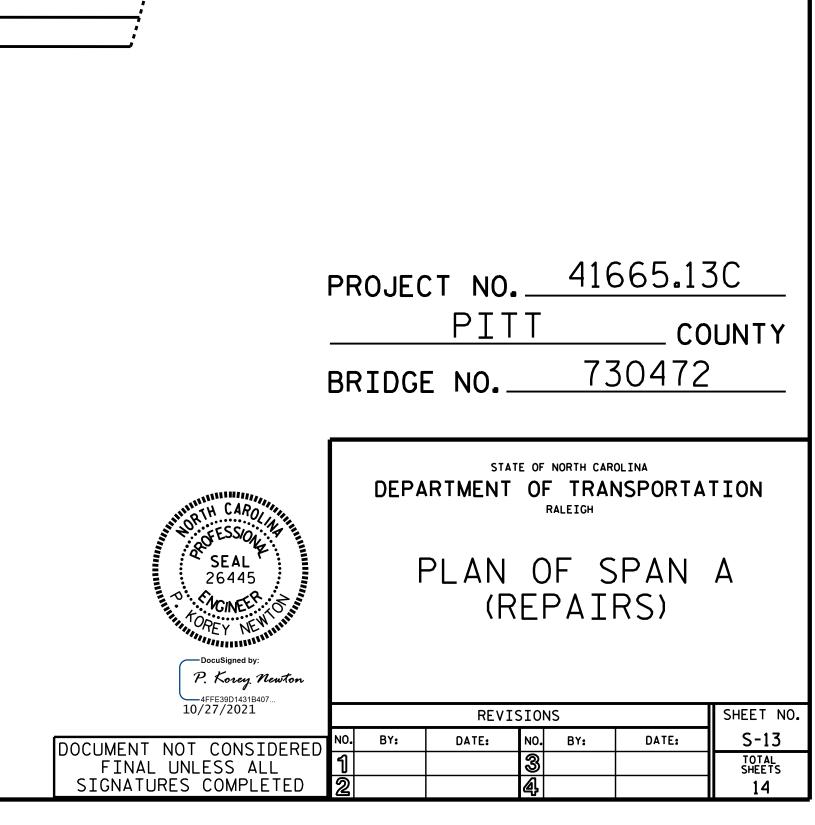
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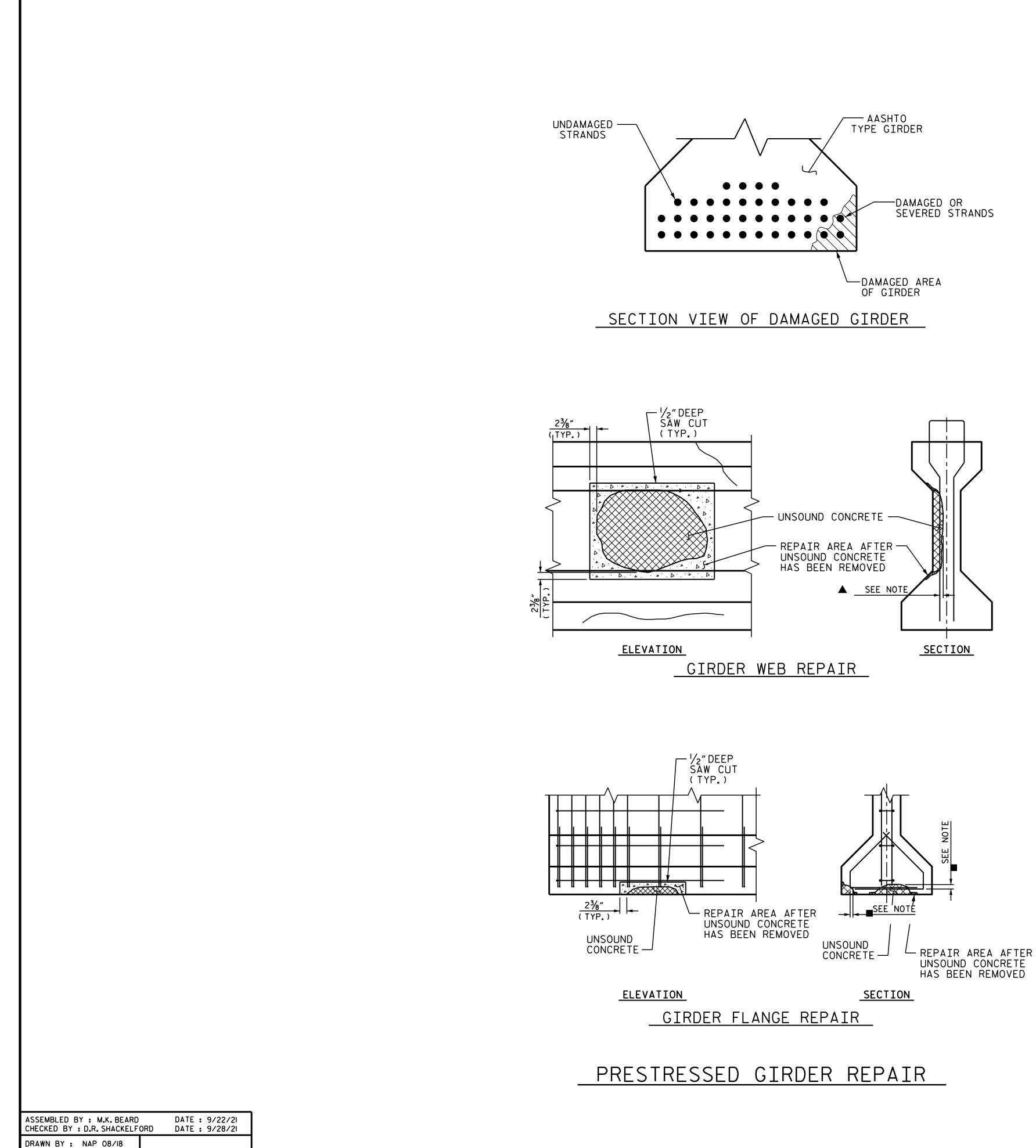


BRIDGE NO.730472
SILANE TREATMENT F
EPOXY RESIN INJECT
REPAIRS TO PRESTRE

DRAWN BY :	M.K. BEARD	DATE : <u>9/21/21</u>
CHECKED BY :	D.R. SHACKELFORD	DATE : <u>9/28/21</u>

REPAIR QUANTITY TABLE					
SPAN A					
	ESTIMATE	ACTUAL			
FOR PRESTRESSED CONCRETE GIRDERS	339.5 SQ. YDS.				
TION	40.0 LIN.FT.				
ESSED CONCRETE GIRDERS	5.8 CU.FT.				





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CHECKED BY :

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PREPACKAGED MATERIAL IS REQUIRED.

CONSULT WITH THE ENGINEER TO DETERMINE PRELOADING REQUIREMENTS WHEN REPAIR IS WITHIN THE CENTER REGION OF THE BEAM (0.25L TO 0.75L).

FOR REPAIRS OVER TRAFFIC AND SHALLOW REPAIRS THAT DO NOT ENGAGE REINFORCEMENT, ANCHOR PATCH MATERIAL USING 1/4" GALVANIZED BOLTS, EPOXY ANCHORED WITH 2"EMBEDMENT. PLACE BOLTS IN A 6"GRID. USE A LATEX OR EPOXY PATCH MATERIAL FOR IMPROVED BOND. USE EXTREME CARE TO NOT DAMAGE STRANDS.

FOR PRESTRESSED CONCRETE GIRDER REPAIRS, SEE SPECIAL PROVISIONS.

FOR EPOXY RESIN INJECTION (ERI), SEE SPECIAL PROVISIONS.

FOR EPOXY COATING CONCRETE GIRDER ENDS, SEE SPECIAL PROVISIONS.

PRESTRESSED GIRDER REPAIR SEQUENCE:

SOUND CONCRETE TO DETERMINE EXTENTS OF REPAIR LOCATION.

REMOVE SURFACE CONCRETE TO VERIFY THAT SAWCUT DEPTH WILL NOT DAMAGE EXISTING REINFORCING STEEL.SAW CUT AROUND REPAIR AREA TO A NOMINAL DEPTH OF 1/2".

REMOVE CONCRETE WITHIN SAW CUT AREA TO MINIMUM $\frac{1}{2}$ " DEPTH. IF CONCRETE IS DAMAGED BEYOND THE ORIGINAL SAW CUT, A NEW SAW CUT IS REQUIRED.

4. IF MORE THAN HALF THE CIRCUMFERENCE OF A REINFORCING BAR IS EXPOSED ▲ DURING THIS PROCESS, REMOVE ADDITIONAL CONCRETE TO 1"BEHIND THE BAR. THIS DOES NOT APPLY TO PRESTRESSED STRANDS.

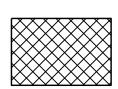
5. ALL UNSOUND CONCRETE MUST BE REMOVED, HOWEVER, PRESTRESSED STRANDS ■ SHOULD NOT BE DISTRUBED UNLESS ABSOLUTELY NECESSARY. USE EXTREME CARE TO NOT DAMAGE STRANDS.

AS INDICATED IN THE SPECIAL PROVISION FOR REPAIRS TO PRESTRESSED COCNRETE GIRDERS, CLEAN ALL EXPOSED REINFORCING BARS AND PRESTRESSED STRANDS. FOR BARS WITH MORE THAN 10% SECTION LOSS, SPLICE AND SECURELY TIE SUPPLEMENTAL REINFORCING BARS AS NEEDED. NOTE AND PROVIDE DETAILED DOCUMENTATION, INCLUDING LOCATION AND SEVERITY, OF ALL DAMAGE TO PRESTRESSED STRANDS THAT EXCEEDS 10% SECTION LOSS. IF FIVE OR MORE STRANDS ARE DAMAGED, NOTIFY THE ENGINEER PRIOR TO PLACEMENT OF REPAIR MATERIAL.

REMOVE ALL LOOSE OR WEAKENED MATERIAL THEN CLEAN THE REPAIR AREA OF DIRT, GREASE, OIL, AND FOREIGN MATTER.

PREPARE SURFACE AND PLACE APPROVED REPAIR MATERIAL ACCORDING TO MANUFACTURER'S RECOMMENDATIONS. MAXIMUM AGGREGATE SIZE FOR REPAIR MATERIAL SHALL NOT EXCEED $\frac{2}{3}$ THE MINIMUM REPAIR DEPTH.

> CRACKS TO BE REPAIRED WITH \sim EPOXY RESIN INJECTION (ERI)



UNSOUND CONCRETE TO BE REPAIRED

	PROJECT NO.	41665 . 13C
	PITT STATION:	COUNTY 730472
WESSION	DEPARTMENT OF	NORTH CAROLINA TRANSPORTATION RALEIGH
Bocusigned by: P. Korey Newton	GIRDEF	ED CONCRETE REPAIR AILS

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SIGNATURES COMPLETED	2			4			14

DESIGN DATA:

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SPECIFICATIONS	A.A.S.H.T.O. (CURRENT)
LIVE LOAD	SEE PLANS
IMPACT ALLOWANCE	SEE A.A.S.H.T.O.
STRESS IN EXTREME FIBER OF STRUCTURAL STEEL - AASHTO M270 GRADE 36	20,000 LBS.PER SQ.IN.
- AASHTO M270 GRADE 50W	27,000 LBS.PER SO.IN.
- AASHTO M270 GRADE 50	27,000 LBS. PER SQ. IN.
REINFORCING STEEL IN TENSION - GRADE 60	24,000 LBS. PER SQ. IN.
CONCRETE IN COMPRESSION	1,200 LBS.PER SQ.IN.
CONCRETE IN SHEAR	SEE A.A.S.H.T.O.
STRUCTURAL TIMBER - TREATED OR UNTREATED EXTREME FIBER STRESS	1,800 LBS.PER SQ.IN.
COMPRESSION PERPENDICULAR TO GRAIN OF TIMBER	375 LBS.PER SQ.IN.
EQUIVALENT FLUID PRESSURE OF EARTH	30 LBS.PER CU.FT. (MINIMUM)

MATERIAL AND WORKMANSHIP:

EXCEPT AS MAY OTHERWISE BE SPECIFIED ON PLANS OR IN THE SPECIAL PROVISIONS, ALL MATERIAL AND WORKMANSHIP SHALL BE IN ACCORDANCE WITH THE 2018 "STANDARD SPECIFICATIONS FOR ROADS AND STRUCTURES" OF THE N.C. DEPARTMENT OF TRANSPORTATION.

STEEL SHEET PILING FOR PERMANENT OR TEMPORARY APPLICATIONS SHALL BE HOT ROLLED.

CONCRETE:

UNLESS OTHERWISE REQUIRED ON PLANS, CLASS A CONCRETE SHALL BE USED FOR ALL PORTIONS OF ALL STRUCTURES WITH THE EXCEPTION THAT: CLASS AA CONCRETE SHALL BE USED IN BRIDGE SUPERSTRUCTURES, ABUTMENT BACKWALLS, AND APPROACH SLABS; AND CLASS B CONCRETE SHALL BE USED FOR SLOPE PROTECTION AND RIP RAP.

CONCRETE CHAMFERS:

UNLESS OTHERWISE NOTED ON THE PLANS, ALL EXPOSED CORNERS ON STRUCTURES SHALL BE CHAMFERED $\frac{3}{4}$ " with the following exceptions: TOP CORNERS OF CURBS MAY BE ROUNDED TO 11/2" RADIUS WHICH IS BUILT INTO CURB FORMS; CORNERS OF TRANSVERSE FLOOR EXPANSION JOINTS SHALL BE ROUNDED WITH A 1/4" FINISHING TOOL UNLESS OTHERWISE REQUIRED ON PLANS; AND CORNERS OF EXPANSION JOINTS IN THE ROADWAY FACES AND TOPS OF CURBS AND SIDEWALKS SHALL BE ROUNDED TO A 1/4" RADIUS WITH A FINISHING STONE OR TOOL UNLESS OTHERWISE REQUIRED ON PLANS.

DOWELS:

DOWELS WHEN INDICATED ON PLANS AS FOR CULVERT EXTENSIONS, SHALL BE EMBEDDED AT LEAST 12" INTO THE OLD CONCRETE AND GROUTED INTO PLACE WITH 1:2 CEMENT MORTAR.

STANDARD NOTES

ALLOWANCE FOR DEAD LOAD DEFLECTION, SETTLEMENT, ETC. IN CASTING SUPERSTRUCTURES:

BRIDGES SHALL BE BUILT ON THE GRADE OR VERTICAL CURVE SHOWN ON PLANS. SLABS, CURBS AND PARAPETS SHALL CONFORM TO THE GRADE OR CURVE.

ALL DIMENSIONS WHICH ARE GIVEN IN SECTION AND ARE AFFECTED BY DEAD LOAD DEFLECTIONS ARE DIMENSIONS AT CENTER LINE OF BEARING UNLESS OTHERWISE NOTED ON PLANS. IN SETTING FORMS FOR STEEL BEAM BRIDGES AND PRESTRESSED CONCRETE GIRDER BRIDGES, ADJUSTMENTS SHALL BE MADE DUE TO THE DEAD LOAD DEFLECTIONS FOR THE ELEVATIONS SHOWN. WHERE BLOCKS ARE SHOWN OVER BEAMS FOR BUILDING UP TO THE SLAB, THE VERTICAL DIMENSIONS OF THE BLOCKS SHALL BE ADJUSTED BETWEEN BEARINGS TO COMPENSATE FOR DEAD LOAD DEFLECTIONS, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER. WHERE BOTTOM OF SLAB IS IN LINE WITH BOTTOM OF TOP FLANGES, DEPTH OF SLAB BETWEEN BEARINGS SHALL BE ADJUSTED TO COMPENSATE FOR DEAD LOAD DEFLECTION, VERTICAL CURVE ORDINATE, AND ACTUAL BEAM CAMBER.

IN SETTING FALSEWORK AND FORMS FOR REINFORCED CONCRETE SPANS, AN ALLOWANCE SHALL BE MADE FOR DEAD LOAD DEFLECTIONS, SETTLEMENT OF FALSEWORK, AND PERMANENT CAMBER WHICH SHALL BE PROVIDED FOR IN ADDITION TO THE ELEVATIONS SHOWN. AFTER REMOVAL OF THE FALSEWORK, THE FINISHED STRUCTURES SHALL CONFORM TO THE PROFILE AND ELEVATIONS SHOWN ON THE PLANS AND CONSTRUCTION ELEVATIONS FURNISHED BY THE ENGINEER.

DETAILED DRAWINGS FOR FALSEWORK OR FORMS FOR BRIDGE SUPERSTRUCTURE AND ANY STRUCTURE OR PARTS OF A STRUCTURE AS NOTED ON THE PLANS SHALL BE SUBMITTED TO THE ENGINEER FOR APPROVAL BEFORE CONSTRUCTION OF THE FALSEWORK OR FORMS IS STARTED.

REINFORCING STEEL:

ALL REINFORCING STEEL SHALL BE DEFORMED. DIMENSIONS RELATIVE TO PLACEMENT OF REINFORCING ARE TO CENTERS OF BARS UNLESS OTHERWISE INDICATED IN THE PLANS. DIMENSIONS ON BAR DETAILS ARE TO CENTERS OF BARS OR ARE OUT TO OUT AS INDICATED ON PLANS.

WIRE BAR SUPPORTS SHALL BE PROVIDED FOR REINFORCING STEEL WHERE INDICATED ON THE PLANS. WHEN BAR SUPPORT PIECES ARE PLACED IN CONTINUOUS LINES. THEY SHALL BE SO PLACED THAT THE ENDS OF THE SUPPORTING WIRES SHALL BE LAPPED TO LOCK LEGS ON ADJOINING PIECES.

STRUCTURAL STEEL:

AT THE CONTRACTOR'S OPTION, HE MAY SUBSTITUTE $\frac{7}{8}$ " Ø SHEAR STUDS FOR THE $\frac{3}{4}$ " Ø STUDS SPECIFIED ON THE PLANS. THIS SUBSTITUTION SHALL BE MADE AT THE RATE OF 3 - 7/8" Ø STUDS FOR 4 - 3/4" Ø STUDS, AND STUD SPACING CHANGES SHALL BE MADE AS NECESSARY TO PROVIDE THE SAME EQUIVALENT NUMBER OF $\frac{7}{8}$ "Ø STUDS ALONG THE BEAM AS SHOWN FOR $\frac{3}{4}$ "Ø STUDS BASED ON THE RATIO OF 3 - $\frac{7}{8}$ "Ø STUDS FOR 4 - $\frac{3}{4}$ "Ø STUDS. STUDS OF THE LENGTH SPECIFIED ON THE PLANS MUST BE PROVIDED. THE MAXIMUM SPACING SHALL BE 2'-O".

EXCEPT AT THE INTERIOR SUPPORTS OF CONTINUOUS BEAMS WHERE THE COVER PLATE IS IN CONTACT WITH BEARING PLATE, THE CONTRACTOR MAY, AT HIS OPTION, SUBSTITUTE FOR THE COVER PLATES DESIGNATED ON THE PLANS COVER PLATES OF THE EQUIVALENT AREA PROVIDED THESE PLATES ARE AT LEAST $\frac{5}{16}$ " in thickness and DO NOT EXCEED A WIDTH EQUAL TO THE FLANGE WIDTH LESS 2"OR A THICKNESS EQUAL TO 2 TIMES THE FLANGE THICKNESS. THE SIZE OF FILLET WELDS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT ANSI/AASHTO/AWS "BRIDGE WELDING CODE". ELECTROSLAG WELDING WILL NOT BE PERMITTED.

WITH THE SOLE EXCEPTION OF EDGES AT SURFACES WHICH BEAR ON OTHER SURFACES, ALL SHARP EDGES AND ENDS OF SHAPES AND PLATES SHALL BE SLIGHTLY ROUNDED BY SUITABLE MEANS TO A RADIUS OF APPROXIMATELY V_{16} INCH OR EQUIVALENT FLAT SURFACE AT A SUITABLE ANGLE PRIOR TO PAINTING, GALVANIZING, OR METALLIZING.

HANDRAILS AND POSTS:

METAL STANDARDS AND FACES OF THE CONCRETE END POSTS FOR THE METAL RAIL SHALL BE SET NORMAL TO THE GRADE OF THE CURB, UNLESS OTHERWISE SHOWN ON PLANS. THE METAL RAIL AND TOPS OF CONCRETE POSTS USED WITH THE ALUMINUM RAIL SHALL BE BUILT PARALLEL TO THE GRADE OF THE CURB.

METAL HANDRAILS SHALL BE IN ACCORDANCE WITH THE PLANS. RAILS SHALL BE AS MANUFACTURED FOR BRIDGE RAILING. CASTINGS SHALL BE OF A UNIFORM APPEARANCE. FINS AND OTHER DEFORMATIONS RESULTING FROM CASTING OR OTHERWISE SHALL BE REMOVED IN A MANNER SO THAT A UNIFORM COLORING OF THE COMPLETED CASTING SHALL BE OBTAINED. CASTINGS WITH DISCOLORATIONS OR OF NON-UNIFORM COLORING WILL NOT BE ACCEPTED. CERTIFIED MILL REPORTS ARE REQUIRED FOR METAL RAILS AND POSTS.

SPECIAL NOTES:

GENERALLY, IN CASE OF DISCREPANCY, THIS STANDARD SHEET OF NOTES SHALL GOVERN OVER THE SPECIFICATIONS, BUT THE REMAINDER OF THE PLANS SHALL GOVERN OVER NOTES HEREON, AND SPECIAL PROVISIONS SHALL GOVERN OVER ALL. SEE SPECIFICATIONS ARTICLE 105-4.



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